# Township of Strong 

## Road Needs Study

Prepared by:
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Date: February 17, 2009

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February 17, 2009
Project Number: 108774 (TSH 3619679)

David Pringle, Roads Superintendent
Township of Strong
1713 Hwy \#11,
P. O. Box 1120, Sundridge, On POA 1 Z0

Dear Mr Pringle;

## Re: Township of Strong

 2008 Municipal Road Inventory Condition AssessmentAECOM is pleased to submit this report with respect to the results of the 2008 Township of Strong Road Needs Study.

The road appraisals in this study were completed using WorkTech's Asset Foundation software and following the methodology of the Inventory Manual, 1991.

With this report, all road related data has been updated to present day values and the content of the report reflects road system conditions as of the time of the field data collection, in the spring of 2008 for the road system. Also included in this study is tabular information reflecting the valuation and accumulated depreciation for the municipal road assets in accordance with the PSAB requirements.

We trust that this report will be beneficial to the Township in developing their asset management plans and wish to express appreciation for the opportunity for AECOM to participate in the work.

Sincerely,

## AECOM Canada Ltd.

## David Anderson, CET

Project Manager, Asset Management
Dave.anderson@aecom.com

## DA:daEncl. <br> cc:

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## Revision Log

| Revision \# | Revised By | Date | Issue / Revision Description |
| :--- | :--- | :--- | :--- |
| 1 | DA | November 2008 |  |
| 2 | DA | November 2008 | I Roger Comments added |
| 3 | DA | January 2009 | Client Comments |
|  |  |  |  |
|  |  |  |  |

## Signature Page

| Report Prepared By: | Report Reviewed By: |
| :--- | :--- |

## Executive Summary

The 2008 Township of Strong Roads Needs Study summarizes roads system surveys conducted during 2008 by identifying road section condition by their time of need and rehabilitation strategy. All of the roads under the Municipality's jurisdiction were included in this survey. Gravel roads were evaluated during mid April in order that spring break-up conditions were observed. Gravel roads are best reviewed during the spring break-up in order that a more accurate assessment of their condition can be determined, however, the performance of each gravel road section can be inconsistent from spring to spring.

The report also includes tabular information reflecting the valuation and accumulated depreciation for the municipal road assets in accordance with the Public Sector Accounting Board (PSAB) requirements, a review of the public works buildings and an equipment replacement schedule

The purpose of a Road Needs Study is to provide an overview of the overall condition of the road system. The study provides a rating of the general condition of the road system, by road section, including such factors as structural adequacy, drainage, and surface condition, as well as providing some indication of what appears to be deficient horizontal and vertical alignment elements per the Ministry of Transportation's manual entitled Geometric Design Standards for Ontario Highways. The study information can be used for programming and budgeting, however, once a road section reaches the project design stage, further detailed review, investigation and design will be required to address the specific requirements of the project. The road needs study is not a road safety audit.

Traffic counts were estimated on the majority of the roads for purposes of this study based on the observations of the rater and/or from information provided by municipal staff. The traffic values established for the purposes of this study are satisfactory. However, from a risk management perspective, the traffic counts entered in the database should not be used to establish road classifications for Minimum Maintenance Standards purposes.

It is recommended that the Municipality have traffic counts undertaken on a larger number of selected roads in order that accurate information is used to factor into decision making and that accurate Road Class determination, as per Regulation 239/02, may be established for maintenance and service level purposes. Accurate traffic counts would be a worthwhile risk management exercise from this perspective alone.

Data collection and road ratings were completed generally in accordance with the MTO Inventory Manual for Municipal Roads (1991)-hereafter referred to as the Inventory Manual or the manual. Road conditions are rated during a field review, and a score calculated which then categorizes the road section as Adequate, or a 'Now', ' 1 to 5 ' or ' 6 to 10 ' year need for reconstruction or resurfacing. Priority ratings are established through a further calculation involving the traffic count and the condition rating. Using the priority rating, data has been further sorted by time of need and rehabilitation strategy. The report summarizes the results of the study through a number of tabular appendices and mapping.

Road sections were created that were consistent throughout their length according to: roadside environment, surface type, condition, cross section, speed limit or a combination of these factors. For
instance, a road section with a hot mix surface that changes from being in good condition to poor condition would require an additional section to be added to the database. Another example would be a road where speed limit changes as it enters a hamlet; a new section would be created to reflect that change even if no other element had changed. Historically, when the province provided funding for municipal road systems, roads systems were measured by their system adequacy. The system adequacy is the percentage of the road system that is not a "NOW" need.

The current system adequacy of the entire Township of Strong road system is $48.18 \%$. (Re-stated, $51.82 \%$ of the roads system is deficient in the NOW time period. The "NOW" needs inventory represents the backlog of work that is required on the road system.) Based on former Ministry of Transportation targets, the minimum target adequacy for the Township of Strong would have been $60 \%$. This calculation does not consider those roads that are deficient that have a traffic count of less than 50 vehicles per day. The Township of Strong Road system includes 16.33 kilometres of road that have an estimated traffic count of less than 50 vehicles per day.

In reviewing the data and the needs for the Township of Strong road network there were several unique aspects of the network that came to light

- A large percentage of roads had definite needs; however, their cross sections in terms of pavement and platform width were adequate for the estimated traffic volumes, which led to a large number of sections ( 42.77 km ) having a recommended rehabilitation strategy of BS—Base and Surface Tolerable. This strategy is recommended where the road structure base is showing signs of distress and/or failure but the cross-sectional elements are satisfactory. This may have been resultant from the original construction methodology, which was typical through a lot of rural Ontario, where ditches were created to provide drainage but the material excavated from the ditch area was used for the road bed. That material is generally not suitable for the road bed as it contains high percentages of organic material and other deleterious material.
- A large number of the rural roads have vertical and horizontal curves that do not provide sufficient visibility for safe stopping sight distance and/or cannot be negotiated at the posted speed limits. The Township of Strong should consider a review of signage to ensure that this risk is mitigated to the greatest extent possible. Appendix F provides a summary of those deficiencies.
- AECOM staff were advised that the speed limits were generally $60 \mathrm{~km} / \mathrm{hr}$ in the rural area. To paraphrase the Highway Traffic Act (HTA), unless otherwise posted, the speed limit on a rural road is $80 \mathrm{~km} / \mathrm{hr}$. Similarly, the HTA provides that the speed limit in built up areas is $50 \mathrm{~km} / \mathrm{hr}$ unless otherwise posted. It did not appear that there was sufficient signage to indicate the municipal standard was $60 \mathrm{~km} / \mathrm{hr}$ in the rural areas and similarly the limits of the $50 \mathrm{~km} / \mathrm{hr}$ zones in built up areas should be denoted. The Township should review this from a risk management perspective and provide appropriate signage.

The Inventory Manual provides direction with respect to roads with a traffic volume of less than 50 vehicles per day are deemed to be adequate, even if they have structural, geometric or drainage deficiencies that would otherwise rate them as having a need. Deficiencies in roads with low traffic values are to be corrected within the maintenance budget. (Roads with less than 50 vehicles per day, and speed limit of less than $80 \mathrm{~km} / \mathrm{hr}$, would be classified as Class 6 roads, as per Regulation 239/02, Minimum Maintenance Standards, which basically do not have a standard for repair.)

This report indicates estimated total cost of improvements for the road system as $\$ 34,974,555$ based on calculations using the Municipality's own benchmark costs. Of those needs, $\mathbf{\$ 2 3 , 2 4 0 , 1 7 1}$ is for those roads that are already deficient (NOW needs). The remaining $\$ 11,734,384$ is for roadworks that are required in the 1 to10 year time period.

Three Public Works Facilities were inspected in August 2008 as part of the project. Required work for the facilities is categorized as essential, required, or desirable. Essential work should be undertaken at the municipality's earliest possible opportunity. The total dollar value of all works required for the facilities is \$336,000.

Generally, lower volume roads in a lower tier road system have deficiencies with the existing horizontal and vertical alignment. These deficiencies are noted within the database. As the Township of Strong develops its asset management plan, which may include rehabilitations in lieu of full reconstructions as interim measures, consideration should be given to those vertical and horizontal elements that may not be corrected as a result of rehabilitation, and should be addressed through other means such as improved signage.

Based on an analysis of the composition of the Township of Strong's road system, to ensure that the road system adequacy is not further reduced, minimum annual capital expenditure levels in the different roads programming areas are as follows:

- $\$ 538,600$ for roads system capital based upon a 50 year life cycle
- $\$ 26,300$ for annual hot mix resurfacing based upon a 20 year life cycle

The above noted program values for the road system do not include any replacement costs for sidewalks, street lighting etc. For structures, the dollar value represents an annualized recommended expenditure.

It should be further noted that the above noted capital recommendations do not include programming that is required due to development growth.

Major maintenance for lower volume roads is also often an area of concern for municipalities, particularly for surface treated and gravel roads. Generally, expenditures in this area are funded from the operating budget. Recommended expenditure levels in those program areas are as follows:

- $\$ 95,900$ annually, for single surface treatment of existing surface treated roads based on a 7 year cycle (This does not include any gravel road conversion costs; those costs would be additional)
- \$681,400 annually for resurfacing gravel roads on a 3 year cycle (This does not include any gravel road conversion costs; those costs would be additional; also does not include ditching, regrading, dust control etc...)

AECOM would recommend that Township of Strong consider a gravel road conversion program. Net Present Value analyses of the gravel and hard topped road maintenance programs generally reveals that over the life cycle (typically 20 years) conversion to a surface treated road is generally more cost effective
than having a gravel road system. Similar analyses also reveal the benefits of moving to a hot mix asphalt-type surface. A number of road agencies in both the United States and Canada have arrived at the same conclusion. Further geotechnical review of candidate roads should be undertaken before advancing the program.

Careful consideration should be given to the pavement/road management strategy especially where funding is limited. Where there are funding constraints, higher priority should be given to those programs that extend the life cycle of the road by providing the correct strategy at the optimum time. As an example, resurfacing and rehabilitation projects should get a higher priority than reconstruction projects, if road program funding is limited.

The Township of Strong's asset management strategy should focus on utilizing the existing funding to preserve/extend the lives of those assets that are appropriate for these strategies. Many studies have proven that it is far less expensive to keep a good road in good condition than it is to reconstruct a road.

The prime goal of any pavement management strategy should be, as an absolute minimum, to maintain the overall system adequacy. The funding level for the road related programming should be set at a sufficient level so as to ensure that the overall system adequacy does not decrease over time.

PSAB 3150, Accounting for Tangible Assets, comes into effect in 2009 and affects all municipalities in how they report on their capital assets by now moving to full accrual accounting, aligning more with private sector practices.

Implementation of PSAB 3150 will improve transparency, openness and accountability of municipal operations. It effectively also introduces a more tangible realization of the gapping in funding that may be occurring for roads related programming (or any other capital asset) and may better quantify the effect of the change in system adequacy.

Whereas, the system adequacy is certainly indicative of the trend occurring with respect to the condition of the road system, once PSAB 3150 is fully implemented, a dollar value can be assigned to that difference in adequacy, clearly highlighting programming or funding inadequacies, or both.

AECOM makes the following recommendations for management of the Township of Strong's road inventory:

1. A regular traffic counting program should be undertaken in order to have accurate traffic counts on all road sections to reduce the exposure to risk for the municipality
2. Substandard vertical and horizontal alignments should be reviewed and considered for additional signage.
3. Roads should be reviewed to ensure that sufficient and appropriate regulatory signage is in place.
4. When road sections are rehabilitated or reconstructed, substandard vertical and horizontal alignments should be addressed either through spot improvements to the alignments or opportunities for additional signage should be reviewed.
5. Given the length of the gravel road network, consideration should be given to developing and implementing a gravel road conversion program.
6. The opportunity to develop a sustainable asset management/financial plan should be reviewed for implementation over a five to ten year period.
7. The condition of the road system should be reviewed on a regular basis to measure the effectiveness of strategies and funding levels.
8. When roads are constructed/reconstructed/rehabilitated the Ontario Provincial Standards and Specifications (OPSS) should be adhered to with particular attention to construction practices, procedures and standards
9. The asset management strategy for the foreseeable future should be developed along the following lines

- The reconstruction program should be deferred over the next few years in favour of preservation activities that extend the life of the existing good road sections.
- Given the existing funding level for roads, the basic strategy should be one of preservation; the top priority is to 'keep the good roads good'
- Optimize the hot mix overlay program and the surface treatment program.
- Ensuring structure inspections are completed every two years.
- If additional funding becomes available, that should be directed toward reconstruction projects.
- Defer replacement of structures that are structurally sound that have a geometric need; improve the signage on those structures in the interim.
- Prioritize structure major maintenance activities such as rehabilitations and resurfacing
- At the time of structure rehabilitations, review the opportunity for conversion to semi-integral type structures


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| :--- | :--- |
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## 1. Background and Introduction

The Township of Strong 2008 Roads Needs Study provides a summary of road condition ratings identified during rating surveys conducted by AECOM, during 2008. All of the Municipality's roads were rated and are included in this report. Gravel roads were evaluated during mid-April in order that spring break-up conditions may be observed. Gravel roads are best reviewed during the spring break-up in order that a more accurate assessment of their condition can be determined, however, the performance of each gravel road section can be inconsistent from spring to spring.

The purpose of the report is to clearly identify the current and future construction and financial needs of the Municipality with respect to its road system. It does not include costing for appurtenant devices or infrastructure such as sidewalks, guiderail and street lighting. The Road Needs Study provides an overview of the overall condition of the road system. The study provides a rating of the general condition of the road system, by road section, including such factors as structural adequacy, drainage, and surface condition, as well as providing some indication of what appears to be deficient horizontal and vertical alignment elements per the Ministry of Transportation's manual entitled Geometric Design Standards for Ontario Highways. The study information can be used for high level programming and budgeting, however, once a road section reaches the project design stage, further detailed review, investigation and design will be required to address the specific requirements of the project.

Traffic counts were estimated on the majority of the roads for purposes of this study based on the observations of the rater or from information provided by municipal staff. The traffic values established for the purposes of this study are satisfactory. However, from a risk management perspective, the traffic counts entered in the database should not be used to establish road classifications for Minimum Maintenance Standards purposes.

It is recommended that the Municipality have traffic counts undertaken on a larger number of selected roads in order that accurate information is used to factor into decision making and that accurate Road Class determination, as per Regulation 239/02, may be established for maintenance and service level purposes. Accurate traffic counts would be a worthwhile risk management exercise from this perspective alone.

Within the body of this report the following information is provided:

- A summary of the road condition ratings, reporting on the results in a tabular format by Road Section, Priority Rating, Time of Need and Rehabilitation Strategy (with associated mapping)
- An overview of the report methodology and evaluation system
- A valuation of the road system
- A Facility Audit of the three Public Works Buildings
- Recommendations for pavement and structure management strategies.
- Recommendations for a gravel road conversion program
- Recommendations for program funding levels.

The Road Needs Study is an important tool for municipalities as it allows municipalities to, in effect, benchmark against themselves, and provides an overview from both programming and financial perspectives.

Structures were not part of this assignment, however, structures form an integral part of the road system. For structures, the Province of Ontario passed amendments in 1997 to existing legislation in the Highway Traffic Act (HTA), The Bridges Act (BA) and the Public Transportation and Highway Improvement Act (PTHIA) that required all bridge and culvert structures with a span greater than 3 m to be inspected under the direction of a Professional Engineer, at no greater than 2 year intervals. The inspection methodology and reporting must be in accordance with the Ontario Structure Inspection Manual (or equivalent). The same high level view of the structures inventory is provided through this exercise, as was for the road inventory.

## 2. Report Content and Scope

The report was prepared by AECOM for the Township of Strong, generally in accordance with the roads condition rating methodology previously prescribed by the MTO in the Inventory Manual for Municipal Roads (1991).

The scope of the report includes summaries of data collected with discussion and analysis on same.

## 3. Report Methodology

### 3.1 Road Condition Ratings

Road section ratings were completed in accordance with the MTO's Inventory Manual for Municipal Roads (1991). The condition ratings, priority ratings, and associated costs were calculated in accordance with the Inventory Manual using WorkTech's Asset Foundation software. Benchmark construction costs were developed from input from municipal staff and AECOM's Bracebridge office.

The network is composed of road sections that are consistent throughout their length according to: roadside environment, surface type, condition, cross section, speed limit or a combination of these factors. For instance, a road section with a hot mix surface that changes from being in good condition to poor condition would require an additional section to be added to the database. Another example would be a road where the speed limit changes as it enters a hamlet; a new section would be created to reflect that change even if no other element had changed.

The Condition Ratings developed through the scoring in the Inventory Manual classify roads as 'NOW', '1 to 5 ', or ' 6 to 10 ' year needs for reconstruction or resurfacing. Field data is obtained through a visual
examination of the road system including structural adequacy, level of service, maintenance demand, horizontal and vertical alignment, surface and shoulder width, surface condition, and drainage. The Condition Rating is calculated based upon a combination of other calculations and data. In the WorkTech software program, further calculations are also made to determine the priority rating which is a function of the Condition Rating and the Average Annual Daily Traffic (AADT). The Priority Rating may be used as a sorting tool within program areas.

The times of need and the 'ADEQ' rating are defined as follows:

### 3.1.1 'Now'

The Now needs inventory generally represents the backlog of work required on the road system. Construction improvements identified within this time period should be undertaken immediately (notwithstanding funding levels and pavement management strategy). It should be noted that a resurfacing strategy is never a 'NOW' need. (The exception being when the surface type is inadequate for the traffic volume.)

If a road with a rehabilitation strategy of "resurface" deteriorates too far, it becomes a 'NOW' construction need. A 'NOW" need rating may be triggered by substandard ratings in any of the Structural Adequacy, Surface Type, Surface Width, Capacity, Drainage, or Geometrics data fields.

Figure 1 "Now" Need Road


The Inventory Manual indicates that roads with a traffic volume of less than 50 vehicles per day will be deemed adequate and deficiencies on those roads are to be corrected with the maintenance budgets. In the case of the Township of Strong, there appears to be 16.33 km of roads with an estimated traffic count of less than 50 vehicles per day. Since the counts are estimated, rather than by an actual count, it would be in the best interests of the municipality to undertake a regular traffic counting program in order that the service classes for the roads are accurately established, for risk management and defensibility purposes.

### 3.1.2 '1 to 5’

' 1 to 5 ' Identifies road sections where construction and resurfacing improvements are anticipated within the next 5 years, based upon a review of their current condition.

Figure 2 "1 to 5" Year Need Road (Resurfacing)


### 3.1.3

Figure 3 " 6 -10" Year Need Road (Resurfacing)

'6 to 10' Identifies improvements expected to be required in the near future.
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### 3.1.4 'ADEQ'

Road section is categorized as adequate by The Inventory Manual rating system. It should be noted that an 'ADEQ' rating encompasses a wide range of conditions.

Figure 4 "Adequate" Road (Approx. 7 year old)


### 3.2 Types of Improvements - Roads

Deficient sections and structures each have an identified improvement type as part of the rating that is conducted.

On a general basis, one of the key factors in making a decision with respect to an improvement type, and in making a determination of whether the appearance and performance of a road relates to an underlying structural problem or simply to aged surface materials, is the visual survey. A road's structural or drainage problem would tend to lead toward a reconstruction/replacement type of strategy; whereas, aged surface materials would tend toward a resurfacing type of strategy. This determination is critical to the Municipality, as reconstructing a road that should have had some type of resurfacing strategy would be an ineffective use of available resources.

Improvement types include the following:

- R1 - Basic Resurfacing
- R2 - Basic Resurfacing -double lift
- RM - Major Resurfacing
- PR1 - Pulverizing and Resurfacing
- PR2 - Pulverizing and Resurfacing -Double lift
- BS - Tolerable standard for lower volume roads
- RW - Resurface and widen
- REC - Reconstruction
- RNS - Reconstruction Nominal Storm Sewers (Urban- no new sewer, adjust Manholes, catchbasins, add sub-drain, remove and replace curb and gutter, granular and hot mix)
- RSS - Reconstruction including installation of Storm Sewers (New storm sewers and manholes in addition to the above)
- NC - Proposed road Construction
- SRR - Storm Sewer Installation and Road re-instatement.


### 3.3 Bridge and Culvert Ratings

Bridge and culvert inspections were not completed by AECOM staff on Township of Strong's structure inventory as part of this study. However, structures are an important integral part of the road system infrastructure and as such this commentary has been included in the report as the management of the structures within the inventory is critical to the overall function of the road system.

Provincial legislation requires that inspections be undertaken on all structures that have a span greater than 3 m in accordance with the OSIM/MBADES inspection manuals. AECOM recommends that these inspections be undertaken by a qualified consultant in order that the municipality demonstrates due diligence in the management of the structure inventory.

Figure 5 "NOW" Need Bridge (from another municipality)

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In general terms, bridges and culverts are defined as follows:

- Bridge: transfers all live loads through a superstructure to a substructure and to the foundations.
- Culverts: transfers all live loads through fill.

Structures are rated as deficient or become NOW needs due to:

- Insufficient width of structure ( 6 m minimum)
- Vertical clearance
- Level of Service (cannot accommodate peak hour traffic)
- Structural Capacity


### 3.4 Types of Improvements -Structures

In the MBADES scoring system there is a similarity to the inventory manual in that a time of need and improvement type are established. The OSIM inspection is more detailed, however the data does not follow that same format.

Unlike the roads inventory, a structure may have more than one improvement type and the analysis is divided into two functional categories of evaluation: Material Condition Rating (MCR) and Performance Condition Rating (PCR)

- REB - Remove Existing Bridge
- RSL - Replace Bridge or Culvert - Same Location
- RSP - Rehabilitate Superstructure
- RIR - Railing Improvement/Replacement
- WSR - Wearing Surface Rehabilitation
- PWP - Patch Waterproof and Pave
- CDS - Concrete Deck Soffit Repairs
- CSS - Coating Structural Steel
- IAG - Installation of Approach guiderail
- OTH - Other

It should be noted that a deficient bridge may have a load posting/restriction. The Highway Traffic Act (HTA) provides for municipalities to pass by-laws to restrict loads on a structure. The load restriction must be based on an engineer's report stamped by two professional engineers. Generally, the load restriction by-laws are only in effect for a maximum of two years, then have to be re-reviewed or improvements undertaken.

## 4. Road Structure

To better understand the content and methodology of this report, an overview of how a pavement structure is designed and functions is required. The majority of municipal roads are a pavement structure
referred to as flexible pavement. As such, the following discussion focuses on flexible pavements. Other pavement structure types include rigid and composite and are more typically found on 400 series highways or on arterial roads of larger urban centres.

### 4.1 Overview of a Typical Flexible Pavement Road Structure

The pavement/road structure transmits the wheel loads of vehicles from the road surface to the road subgrade (or native soil). The pavement structure has to be designed such that the load that is transmitted to the sub-grade, is not greater than the sub-grade's ability to support the load. Figure 6 and the accompanying table shows a typical flexible pavement structure.

Figure 6 Typical Wheel Load Stress Distribution for a Flexible Pavement
Source: MTO Soils Manual circa mid 1960's

$\begin{array}{ll}\text { Table } 1 & \begin{array}{l}\text { Typical Wheel Load Stress Distribution for a Flexible Pavement, Source: MTO Soils } \\ \text { Manual circa mid 1960's }\end{array}\end{array}$

| Depth Below Surface | Stress (psi) | Stress (Kpa) |
| :--- | :--- | :--- |
| At surface | 90 | 620.5 |
| $8^{\prime \prime}(200 \mathrm{~mm})$ Below | 11 | 75.84 |
| $11^{\prime \prime}(275 \mathrm{~mm})$ Below | 7 | 48.26 |
| $16^{\prime \prime}(400 \mathrm{~mm})$ Below | 4 | 27.58 |

The highest loading is experienced at the point of contact with the vehicle's tire. With modern radial truck tires that run inflated to 110 psi , the loads at the road surface can be over 20 times higher than at the compacted sub-grade. Figure 6 is a profile view of the way in which the load is distributed through the pavement structure. The loading actually occurs in a conical fashion, dissipating both vertically and
horizontally as it passes through the pavement structure, with the highest loading occurring at the point of contact. Loading decreases exponentially as it passes through the road structure. Therefore materials of lesser strength or lesser quality can be used deeper in the road structure. Re-stated, the closer to the surface of the road the road building materials are placed, the higher the quality of road building materials required. Similarly, the poorer the sub-grade or native material, the deeper/stronger the road structure has to be to carry the same loads.

### 4.2 Drainage

It has often been stated that the 3 most important elements of road building are drainage, drainage and drainage. Proper drainage is imperative in order to maximize the long-term performance of the road structure. Roads are designed, constructed and maintained in order to minimize the amount of water that may enter the road structure.

When water enters a road pavement structure a number of reactions can occur. In summer, the granular road base can become saturated and when too much water displaces the granular material, it removes the material's ability to support the loads it was designed for. Too much water in the granular material actually acts like a lubricant and facilitates the displacement of the material under load. In winter, water in the road structure can cause frost heave, potholes and pavement break-up as the water expands as it freezes. Generally, a saturated granular road base results in structural failure of the road.

Figure 7 OPSS 200.10


Rural road drainage is generally achieved through roadside ditches. The ditches should be a minimum of 500 mm below the granular road base to ensure that the road base remains free from moisture and maintains its ability to carry loads.

Figure 8 Inadequate Roadside Drainage


The side slopes of the ditches are also critical to the stability of the road platform. The drawing indicates a 3:1 side slope and this would be ideal. In most cases a 2:1 slope would also be satisfactory. When slopes are too steep the soil will move over time and find its natural angle of repose. The movement of the soil will contribute to an early failure of the pavement structure. Inadequate compaction will also be a contributing factor to early failure. The following pictures illustrating the steep side slope were not taken in the Township of Strong, but illustrate the longer term effect of a road structure with side slopes that are too steep.

Figure 9 Steep Side Slopes -to be avoided in new construction


Maintenance of the drainage system(s) is also critical to the long term performance of the road system. Low volume rural roads tend to have a winter maintenance program that includes the application of sand to improve traction. Over time that sand builds up on the edge of the pavement to a point where it effectively blocks the runoff from getting to the ditch. The runoff is trapped at the edge of pavement where it saturates that area of the road bed contributing to the early failure of the edge of the pavement.

Figure 10 Shoulder Berm contributing to edge failure


Urban roads have a storm sewer pipe network that carries the minor storm event. The roadway itself is often part of the overland flow route for the major event. The drainage of the granular road base is accomplished through sub-drains installed below the curb and gutter, lower than the lowest elevation of the granular base.

### 4.3 Horizontal and Vertical Alignments

Horizontal and vertical Alignment are the changes in direction and elevation of the road. A large number of roads in rural Ontario, more so further north, were originally constructed along the alignments of the trails from the original settlement of the area. As such they tend to closely follow (or avoid) the existing contours of the land. (In southern Ontario there is a greater tendency to follow the alignments of the original Township surveys due largely to the much flatter landscape.) The result is a road alignment that tends to change vertical and horizontal direction frequently. Those changes generally do not provide sufficient visibility for Safe Stopping Distance (SSD) from the posted speed limit as per the manual entitled Geometric Design Standards for Ontario Highways.

The following table is an excerpt from the Geometric Design Standards for Ontario Highways and indicates the SSD's required for various design speeds.

Table 2 Minimum Stopping Sight Distance on Wet Pavement

Table C2-1
MINIMUM STOPPING SIGHT DISTANCE ON WET PAVEMENTS

| Speed $v$ |  | Perception and Brake <br> Reaction |  | Coefficient <br> of friction <br> wet pav't | Braking <br> distance <br> on level | S-Min. Stopping <br> sight distance |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Design | Assumed <br> condition | Time | Distance |  | rounded |  |  |
| $\mathrm{km} / \mathrm{h}$ | $\mathrm{km} / \mathrm{h}$ | s | m | $f$ | m | m | m |
| 40 | 40 | 2.5 | 28 | 0.380 | 17 | 45 | 45 |
| 50 | 50 | 2.5 | 35 | 0.358 | 27 | 62 | 65 |
| 60 | 60 | 2.5 | 42 | 0.337 | 42 | 84 | 85 |
| 70 | 70 | 2.5 | 49 | 0.323 | 60 | 109 | 110 |
| 80 | 79 | 2.5 | 55 | 0.312 | 79 | 134 | 135 |
| 90 | 87 | 2.5 | 60 | 0.304 | 98 | 158 | 160 |
| 100 | 95 | 2.5 | 66 | 0.29 | 120 | 186 | 185 |
| 110 | 102 | 2.5 | 71 | 0.290 | 141 | 212 | 215 |
| 120 | 109 | 2.5 | 76 | 0.283 | 165 | 241 | 245 |
| $130^{*}$ | 116 | 2.5 | 81 | 0.279 | 190 | 271 | 275 |
| $140^{*}$ | 122 | 2.5 | 85 | 0.277 | 211 | 296 | 300 |
| $150^{*}$ | 127 | 2.5 | 88 | 0.273 | 232 | 320 | 320 |
| $160^{*}$ | 131 | 2.5 | 91 | 0.269 | 251 | 342 | 345 |

*Design Speeds above $120 \mathrm{~km} / \mathrm{h}$ are beyond the normal range of application

It would be unrealistic to expect that all substandard alignments could be removed from all roads in a lower tier roads system, particularly those with lower traffic volumes. However, in order to reduce the exposure to risk for the municipality, those road sections with substandard alignments should be reviewed for installing additional advisory signage.

Figure 11 Substandard Vertical Alignment


Figure 12 Substandard Horizontal Alignment


### 4.4 Pavement Maintenance and Life Cycle

Pavement structure life expectancy will vary dependant on a number of factors including:

- adequacy of initial design
- adequate maintenance programming
- adequate drainage
- traffic volumes
- traffic type

A conventionally designed and constructed flexible road pavement structure for an arterial road should last at least 40 years before it needs to be reconstructed. During that 40 year life span there will be 2 or 3 hot mix overlays required. A local road, carrying less traffic volume and substantially less truck loads, should last at least 50 years before full reconstruction is required. Again, 2 or 3 overlays will be required within this life span. Proper maintenance programming will maximize these life expectancies.

Maintenance programs should include the following components:

- spot improvements to the asphalt surface
- spot improvements to the road drainage system
- crack sealing
- Resurfacing/overlays at the appropriate time.
- Pavement preservation strategies if appropriate:
- Microsurfacing
- Surface Treatment
- Slurry Seals
- Reclamite

More recently, the concept of perpetual pavements has been the topic of discussion at conferences and seminars. The pavement structure design is different from a conventional flexible pavement design in that it generally requires a greater depth of asphalt. This results in a road structure that is less susceptible to fatigue failure.

The goal of perpetual pavement is to provide a pavement structure that is designed and maintained such that only the top layer of the existing asphalt would ever be replaced/rehabilitated over longer life cycle period. The top layer of asphalt is a 'replaceable' wearing surface that protects the underlying road structure, maintaining its structure in perpetuity. An award winning example of perpetual pavement in Ontario is the Don Valley Parkway; constructed in the 1950's and 1960's with only resurfacing being done since its construction.

Given the nature of urban roads and the number of other utilities occupying the road allowance, the perpetual pavement concept may lend itself more easily to rural roads. (Urban roads tend to have an increased number of utility cuts and repairs). Initial construction costs of perpetual pavements will be higher; however, they will be more cost effective on a life cycle basis.

Optimal timing of maintenance and rehabilitation efforts is the key to maximizing life expectancy of the existing pavement structure. A number of road agencies and institutions have developed deterioration curves and/or graphical depictions that illustrate the life cycle of a pavement structure.

Figure 13 Impact of Different Maintenance Strategies on Pavement Performance


Source: Development of a new asphalt pavement performance prediction model; Ningyuan Li, Ralph Haas and Wei-Chau Xie

Figure 14 Alternative Maintenance Strategies


Source: Wirtgen Cold Recycling Manual

The message, consistent with both graphs in Figures 13 and 14, is that timely, appropriate maintenance and rehabilitation extends the life expectancy of the pavement structure.

Timing of major maintenance such as an overlay is dependant upon the purpose of the road and can vary from 12 to 25 years. However, on average an arterial road requires resurfacing at an age of 16 to 20 years. Other studies have indicated that 17 years is the optimal time interval for resurfacing.

## 5. REGULATORY AND ADVISORY SIGNAGE

Most municipal road systems have a significant number of signs advising the road user of various aspects of the road section.

Regulatory signage provides advice to the motoring public on regulatory requirements such as speed zones, and stop, and yield requirements. Provincial legislation such as the Highway Traffic Act provides municipalities with the authority to create speed zones and stop and yield conditions. A municipal by-law must be passed by the Council of the municipality to be able to enforce the regulations.

Warning or Advisory signage provides advice to the motoring public on recommended speeds for substandard corners, hazards, areas or reduced visibility etc.

The following are excerpts from the Ontario Traffic Manual (OTM) which further explain signage;
"The Highway Traffic Act(HTA) Section 182 (R.S.O 1990), provides for the regulation of various signs, their type and location on the roadway. The criteria and specifications for application, dimensions, location and orientation are prescribed and illustrated under Regulations 615,608, 581 and 599 (R.R.O. 1990) and are indicated as such in this manual. Signs erected in accordance with the Regulations, and pursuant to the Highway Traffic Act, are enforceable under various provisions of the Act. Enforcement is
permitted under the particular section under the authority of which a prescribed sign may be erected to indicate a traffic regulation or HTA Section 182 (R.S.O. 1990), which requires obedience to prescribed signs."
"Regulatory signs are signs which inform the driver/road user as to things they should or must do (or not do) under a given set of circumstances. They often indicate traffic regulations which apply at any time (or at times specified) or place upon a street or highway, disregard of which may constitute a violation. They may be supported (1) by the Highway Traffic Act or its regulations, (2) by municipal by-law or (3) not at all. In the first two cases the signs are enforceable; in the third case, although the signs advise road users as to what they should do, they are not enforceable"

The foregoing is a very brief overview of signage and how it is used by a municipality. For more detailed information and guidance, the municipality should obtain copies of the manuals and/or seek advice from an appropriately qualified consulting firm.

To paraphrase the Highway Traffic Act, where regulatory speed signs have not been placed in a rural setting, the speed limit shall be assumed to be $80 \mathrm{~km} / \mathrm{hr}$. Where regulatory speed signs have not been placed in an urban area the speed limit shall be assumed to be $50 \mathrm{~km} / \mathrm{hr}$. This is significant in that, if the roads are not appropriately signed, or if there is not an appropriate by-law in place, the speed limits are not enforceable, and the roadway classifications for purposes of Regulation 239/02 will be inaccurate, creating additional exposure to risk for the municipality.

When AECOM staff were conducting their inspections of the road system they had assumed that a number of sections were an $80 \mathrm{~km} / \mathrm{hr}$ speed zone in the absence of signage. Subsequently AECOM was advised by municipal staff that the rural roads were $60 \mathrm{~km} / \mathrm{hr}$ speed zones. Although there were a number of $60 \mathrm{~km} / \mathrm{hr}$ signs it did not appear that there was sufficient signage to indicate the municipal standard was $60 \mathrm{~km} / \mathrm{hr}$ in the rural areas and similarly the limits of the 50 km zones in built up areas should be denoted. The Township should review this from a risk management perspective and provide appropriate signage.

Throughout the municipality, there a number of substandard vertical and horizontal curves, which are not signed. The municipality should review the substandard alignment for additional signage to reduce the exposure to risk. Additionally, when the municipality undertakes any rehabilitative or reconstructive activities on a road section, those sub-standard elements should be dealt with either through improved alignment or additional signage, to reduce the exposure to risk.

Regulatory signage that is installed, but not visible or obscured also poses a liability to the municipality.

Figure 15 Obscured Regulatory Signage


Speed Limit Sign
Stop Sign
As part of the road inspection process, signage should be reviewed for visibility

## 6. Township of Strong Road System Inventory and Classification

### 6.1 Surface Type and Roadside Environment

The Township of Strong is classified as an Urban and Rural lower tier road system. Tables 3 \& 4 provide information of the composition of the road system by surface type and by roadside environment. Maps 1 \& $\mathbf{2}$ of this report provide a graphical representation of the information in the tables.

Table 3 indicates that the road surface types throughout the Municipality are composed of primarily three types, these being Gravel, Low Cost Bituminous (Surface Treatment) and High Cost Bituminous (asphalt). Map 1 shows the road network by road surface type.

Table 4 shows that Township of Strong has a largely rural road system with $93.61 \%$ of the road sections having a rural roadside environment. The remainder is semi-urban roadside environments. Map 2 shows the Township of Strong Road System by roadside environment.

Rural Roads- areas of sparse development or where development is less than $50 \%$ of the frontage including developed areas extending less than 300 m on one side or 200 m on both sides, with no curbs and gutters.

Semi-Urban Roads- areas where development exceeds $50 \%$ of the frontage for a minimum of 300 m on one side or 200 m on both sides and no curbs and gutters, with or without storm/combination sewers or for subdivisions where the lot frontages are 30 m or greater.

Urban Roads - Curb and gutter on both sides served with storm or combination sewers or curb and gutter on one side served with storm or combination sewers or reversed paved shoulders with, or served by, storm or combination sewers; subdivisions with frontages less than 30 m .

Table 3 System Breakdown by Surface Type

| Surface Type | Length (km) | Length (\%) |
| :--- | :---: | :---: |
| Earth | 3.59 | 2.38 |
| Gravel (G/S) | 105.47 | 69.84 |
| Low Cost Bituminous (LCB) (Surface Treatment) | 39.04 | 25.85 |
| High Cost Bituminous (HCB) Hot Mix Asphalt | 2.91 | 1.93 |
| Totals | 151.01 | 100 |

Roads are further classified by classes such as Local, Collector, Arterial and Residential or Industrial within the database.

## Table 4 System Breakdown by Roadside Environment

| Roadside Environment | Length (km) | Length (\%) |
| :--- | :---: | :---: |
| Rural (R) | 141.36 | 93.61 |
| Semi-urban (S) | 9.65 | 6.39 |
| Urban (U) | 0 | 0 |

### 6.2 Boundary Roads

Boundary roads by definition are roads that a municipality would have in common with the abutting municipality and generally involve a Boundary Road Agreement which identifies the responsibilities of both agencies. The agreements are usually in writing, however some are informal.

Boundary Road agreements are useful when costs are identified for maintenance or capital work on the road section, and from a risk management perspective they reduce the risk for one of the parties in the event of a claim, dependant upon the content of the agreement.

When a boundary is reconstructed on a day labour basis by the adjacent municipalities, the project should be treated no differently than if the work were being tendered. The exposure to risk for the municipality is no different. The assignment of the various aspects of the work should be clear and the timing for completion of the tasks clearly identified and adhered to.

## Table 5 Boundary Road Lengths (All External)

| Adjacent Municipality | Road Name | From | To | Length |
| :---: | :---: | :---: | :---: | :---: |
| Town of Sundridge | Albert Street | HIGHWAY 11 | Dunbar Street | 0.31 |
| Town of Sundridge | Albert Street | Dunbar Street | Oakridge Drive | 0.38 |
| Municipality of Magnetawan | Chapman Strong Road | Brookside Road | Bloomfield Road | 2.01 |
| Municipality of Magnetawan | Chapman Strong Road | South End | Brookside Road | 1.06 |
| Township of Machar | Machar Strong Boundary Road West ROAD WEST | Old Muskoka Road South | 1.15KM West of Albert Road | 1.71 |
| Township of Machar | Machar Strong Boundary Road West | Morris Lane | Old Muskoka Road South | 3.07 |
| Township of Machar | Machar Strong Boundary Road West | Cheer Lake Road | Park Road South | 1.63 |
| Township of Machar | Machar Strong Boundary Road West | Park Road South | Morris Lane | 0.65 |
| Township of Machar | Machar Strong Boundary Road West | 1.15KM West of Albert Road | Albert Street | 1.15 |
| Township of Machar | Machar Strong Boundary Road West | Albert Street | Tower Road | 2.04 |
| Township of Armour | Pevensey Road | HIGHWAY 11 | Muskoka Road | 1.18 |
| Township of Armour | Pevensey Road | Muskoka Road | 2.8km East | 2.8 |
| Township of Joly | Schmidts Road | Kent Mills Road |  | 0.73 |
| Township of Armour | South Horn Lake Road | Magnetewan Boundary | Rodeo Road | 1.16 |
| Township of Armour | South Horn Lake Road | Rodeo Road | Armour Boundary | 0.21 |
| Township of Armour | Sterling Creek Road | HIGHWAY 11 | West End | 0.35 |
| Township of Joly | Strong Joly Road | Peacock Road | 175 M North of Forest Lake Road | 1.86 |
| Township of Joly | Strong Joly Road | Airport Road | Railway Tracks/Machar Boundary | 1.01 |
| Township of Joly | Strong Joly Road | Airport Road | PEACOCK ROAD | 0.99 |
| Township of Joly | Strong Joly Road | Forest Lake Road | 175 M North of Forest Lake Road | 0.18 |
|  |  |  | TOTAL | 24.48 |

Reporting on boundary roads can be dealt with in one of two ways: their length could be split and provide a more accurate depiction of the road system that is actually maintained by the agency, or they may remain with no adjustment. When MTO was providing subsidy, typically the roads were adjusted. For the purposes of this report no adjustment has been made to the road system sizes to account for the $50 \%$ sharing of the length of the boundary roads.

### 6.3 Road System Value (Excluding Structures)

Section 6 of this report identified the road system breakdown by surface type and by roadside environment. Table 6 (below) provides a conservative estimate of road replacement costs by those parameters on a per kilometre basis. The costs have been prepared based on weighted average widths of each surface type from the municipal database. The values shown in Table 6 include the construction costs based on Township of Strong's unit costs obtained from recent contracts, and adjustment factors including basic construction, contingency, engineering, and terrain type.

In rural lower tier municipalities it is common practice to reconstruct or rehabilitate roads on a 'day labour' basis using the municipality's own staff. In order to accurately compare those costs with the costs of using contracted services, it is imperative that the full costs of the municipal operation be accounted for. Those costs should include;

- Equipment time
- Hourly rates should include fuel, maintenance, repair and capital depreciation
- Manpower costs for Labour and Engineering
- Hourly rates should include benefits, overhead for housing
- Materials
- Unit costs for materials such as aggregate should include complete pit operation costs, equipment rentals, site restoration, land costs


## Table 6 Road Replacement Costs per Kilometre (Existing System)

| Surface Type and Roadside Environment | Replacement Cost per Kilometre |
| :---: | :---: |
| Gravel-Rural | $\$ 143,600$ |
| Surface Treated (Low Cost Bituminous) - Rural | $\$ 260,200$ |
| Hot Mix (High Cost Bituminous) - Rural | $\$ 425,100$ |
| Hot Mix (High Cost Bituminous) - Semi- Urban | $\$ 494,400$ |

Based on the above noted per kilometre costs, a conservative estimate of the replacement value of Township of Strong's road system is $\$ 26,932,175$ as it exists today. (This estimate includes contingencies and engineering, but not removals.) Appendix E of this report includes the parameters used to develop the value of the Township of Strong road system. The road replacement costs noted in Table 6 are estimated generally in accordance with the Inventory Manual and include adjustment factors for basic construction, contingency, engineering, terrain, and roadside environment. The adjustment factors can add from $18 \%$ to over $50 \%$ to the construction costs based on the site specific circumstances.

### 6.4 Public Works Facilities Inventory

AECOM facilities staff inspected two Township of Strong Public Works Facilities in August 2008. The detailed inspection reports can be found in Appendix H of this study.

The following table summarizes the value of the improvements required to the facilities. Essential items reflect costs of Life/Safety needs.

Table $7 \quad$ Facility Replacement and Needs Summary

| Facility | Replacement Cost | Essential | Required | Desirable | Totals |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Sundridge Shed | 80,000 | ---------- | \$14,000 | ----------- | 14,000 |
| Sundridge Garage | \$430,000 | \$2,500 | \$119,000 | \$50,500 | \$172,000 |
| Sundridge Dome | \$330,000 | --------- | \$150,000 | ----------- | \$150,000 |
| TOTALS | \$840,000 | \$2,500 | \$283,000 | \$50,500 | \$336,000 |

### 6.5 Road System Value- PSAB 3150 Implications

PSAB 3150, Accounting for Tangible Assets, comes into effect in 2009 and affects how all municipalities will report on their capital assets. Implementation of PSAB 3150 will improve transparency, openness and accountability of municipal operations.

Until 2009, municipalities will report on their capital assets as they always have, in the years that the funds are expended.

Beginning in 2009, municipalities will have to report on the amortized value of all of their capital assets. PSAB Section 3150 requires that all tangible capital assets be accounted for and amortized on the books of accounts effective January 1, 2009. This is consistent with the move from the current modified accrual basis of accounting toward full accrual accounting for municipalities. Not only will this allow for the full reflection of the costs of using these assets, it will serve to provide for a move toward comparability with other entities.

Municipalities must review the historical costs of assets and determine the residual value. One of the simpler approaches to determine the residual value is to calculate the current replacement value of the road system and then discount the current value based upon an estimate of the age of the road sections.

Table 8 PSAB 3150 Summary for Roads

| Road System | Length | Replacement <br> Cost | Historic Cost | Accumulated <br> Depreciation | Net Book Value |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Hot Mix Asphalt Roads | 2.91 | $\$ 1,326,872$ | $\$ 806,419$ | $\$ 446,921$ | $\$ 359,498$ |
| Surface Treated Roads | 39.04 | $\$ 10,218,295$ | $\$ 3,182,629$ | $\$ 2,047,387$ | $\$ 1,135,243$ |
| Gravel Roads | 105.47 | $\$ 15,330,095$ | $\$ 7,702,297$ | $\$ 3,349,537$ | $\$ 4,352,760$ |
| Earth Roads | 3.59 | $\$ 56,913$ | $\underline{\$ 6,989}$ | $\mathbf{\$ 6 , 9 8 9}$ |  |
|  | $\mathbf{1 5 1 . 0 1}$ | $\mathbf{\$ 2 6 , 9 3 2 , 1 7 5}$ | $\mathbf{\$ 1 1 , 6 9 8 , 3 3 4}$ | $\mathbf{\$ 5 , 8 5 0 , 8 3 3}$ | $\mathbf{\$ 5 , 8 4 7 , 5 0 0}$ |

### 6.6 AECOM PSAB 3150 Model Overview

## Overview

The AECOM PSAB 3150 valuation model for roads and structures establishes accrued deprecation for assets using data commonly available in municipal road inventory databases. The model contains a methodology to address the fact that many road inventory databases do not contain construction date information for older infrastructure. The model also provides an opportunity to separately value the road base and road surface for hot-mix asphalt road sections where the two components are of significant value and have different life spans. Most structure inventories do contain a construction date, therefore determination of historic construction date is a non issue.

The following section describes the valuation process, which is based on methodologies developed by AECOM with reference to available documentation including:

- Assessment of Tangible Capital Assets: Statement of Principles prepared by the Public Sector Accounting Board (2008); and
- Municipal Guide To Accounting For Tangible Capital Assets Version 2 prepared by the Ontario Municipal Benchmarking Initiative (2008).


## Valuation Process

## Step 1: Estimation of Current Replacement Costs

A basic construction cost is estimated for each road section based on current unit prices and quantities. Quantities are calculated with consideration given to the following factors:

- Standard design criteria (i.e. granular depths, asphalt depths, etc.). The design criteria employed are adapted from those found in the Inventory Manual for Municipal Roads, MTO (1991).
- Existing road section geometry (length, surface width and platform width).
- Existing cross section/roadside environment type (rural, semi-urban or urban).

Given that the cost being estimated is the cost to replace the existing asset and not the cost to improve the asset to meet current requirements, no consideration is given to desired geometry or standards. Calculations of this nature are generally performed as part of a road needs study exercise.

For structures, a basic construction cost is estimated based on current unit prices and costing experience. Quantities are calculated based on the 'footprint' of the structure and its' construction type, material, function, area of the province, depth and roadside environment.

## Step 2: Establishing Historic Construction Dates

Construction dates for each road section are established from available records or through estimating based on the condition of the road. The condition is determined by developing a Pavement Condition Index (PCI) value that is dependant upon the surface condition and structural adequacy of the respective
section. The current condition of the road section reflects the approximate age the road which, using the formulae that AECOM has developed, allows a year of construction to be estimated for the road base, and where appropriate, the road surface.

Figure 16 Deterioration Curve


## Step 3: Estimation of Historic Construction Costs

Historic construction costs are estimated by deflating the current replacement cost of each asset. Deflation calculations are based on upon deflation factors obtained from a cost index. Specifically, the appropriate deflation factor for the asset year of construction can be obtained from a number of construction price indices based on data available from the Engineering News Record (ENR), the Federal government and other sources. The data available from the ENR provides deflation factors from 1955 through to present. Alternately the municipality may have elected to use another deflation index such as the NRBCPI index for Construction Assets from the MFOA/AMCTO Deflation Report. This model uses the NRBCPI Index.

## Step 4: Depreciation Calculation

Straight line depreciation formulae are used to develop the accumulated depreciation, and subsequently the Net Book value. The calculation relies on the previously determined construction date, which provides the useful life remaining and the associated life expectancy.

## General Parameters

The following provides an overview of the major parameters that are used within the model.

## Life Expectancies

Tables 9 and 10 below present standard life spans for the surface and base components of common types of road sections and structures. The information presented below is based upon AECOM's
experience and information presented in previously accepted PSAB calculations and can be varied as required to meet specific municipal needs.

Table 9 - Road Life Expectancies by Surface Type

| Surface Type | Road Base Life Span | Surface Life Span |
| :--- | :--- | :--- |
| Hot Mix Asphalt | 40 yrs | 20 yrs |
| Surface Treatment | 40 yrs | n/a (life span is short and <br> replacement is typically <br> considered an operating cost.) |
| Asphalt Over Concrete | 40 yrs | n/ayrs (concrete base acts as <br> Concrete |
| Gravel | 50 yrs | n/a (no surface, gravel <br> resurfacing costs typically <br> considered an operating cost) |

## Cost Estimation Items

Replacement cost estimations are based upon the items presented in Table 3 below. Unit costs for each item can be entered into the model and varied as required to meet specific municipal needs.

Table 10 Cost Estimating Items -For Roads

| Item | Road Side Environment |  |  |
| :--- | :---: | :---: | :---: |
|  | Rural | Semi-Urban | Urban |
| Road Bed Excavation | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Ditch Excavation | $\checkmark$ |  |  |
| Granular 'A' | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Granular 'B' | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Storm Sewer (525mm) ${ }^{1}$ |  |  | $\checkmark$ |
| Maintenance Holes ${ }^{1}$ |  |  | $\checkmark$ |
| Catch Basins ${ }^{1}$ |  |  | $\checkmark$ |
| Curb and Gutter |  |  | $\checkmark$ |
| Sub Drain |  |  | $\checkmark$ |
| Hot Mix Asphalt ${ }^{2}$ |  |  | $\checkmark$ |
| Surface Treatment ${ }^{2}$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| ${\text { Concrete Base }{ }^{2}}$ |  | $\checkmark$ | $\checkmark$ |

Notes:

1. Items can be removed if storm sewer infrastructure is to be separately inventoried and valued. Estimate is for 525 mm diameter sewer under 60 percent of the section's length with catch basins and maintenance holes spaced every 90 m . Design criteria can be varied if required.
2. Inclusion of items depends on the existing surface type of the road section.
3. No removal costs are included in the calculation.
4. Allowances for engineering, construction contingencies and terrain are included in all cost calculations in accordance with standards presented in the Inventory Manual for Municipal Roads, MTO (1991).

## 7. Road System Time of Need and Adequacy

This section of the report will provide two key pieces of information that have been extracted from and/or calculated from the information collected: Time of Need and System Adequacy.

The tabular information provided in the Time of Need section indicates the dollar value of the backlog of work that should be undertaken and provides an estimate of the work that should be undertaken within the typical capital planning horizon for most municipalities. Cost estimates for the work required are generated by the pavement management software based on road type, class, and current unit costing and such will vary considerably on a section by section basis.

The System adequacy calculation will provide a report card on the adequacy or appropriateness of the road programming since the last roads needs study. A decrease in the system adequacy reflects inadequate funding or an inappropriate pavement management strategy.

This report indicates estimated total cost of improvements for the road system as $\$ 34,974,555$ based on calculations using the Municipality's own benchmark costs. Of those needs, $\mathbf{\$ 2 3 , 2 4 0 , 1 7 1}$ is for those roads that are already deficient (NOW needs). The remaining $\$ 11,734,384$ is for roadworks that are required in the 1 to10 year time period.

### 7.1 Time of Need -Roads

Table 11 Summary of Costs by Time of Need (Including Contingencies and Engineering)

| Item | NOW | $\mathbf{1}$ to 5 | 6 to 10 | Total |
| :---: | :---: | :---: | :---: | :---: |
| Construction Needs | $23,240,171$ | $6,786,652$ | $4,835,717$ | $34,862,540$ |
| Resurfacing Needs | ----------- | 60,409 | 51,606 | 112,015 |
| Totals | $\mathbf{\$ 2 3 , 2 4 0 , 1 7 1}$ | $\mathbf{6 , 8 4 7 , 0 6 1}$ | $\mathbf{\$ 4 , 8 8 7 , 3 2 3}$ | $\mathbf{\$ 3 4 , 9 7 4 , 5 5 5}$ |

Maps 5 \& 6 of this report show the road system by time of need. The costs shown in Table 11 include adjustment factors of basic construction, and engineering.

It should be noted that the Inventory Manual provides direction that roads with a traffic volume of less than 50 vehicles per day are deemed to be adequate, even if they have structural, geometric or drainage deficiencies that would otherwise rate them as having a need. Deficiencies in roads with low traffic values are to be corrected within the maintenance budget.

Table 13 indicates the value of the total construction needs to be $\$ 34,974,555$ compared to an estimated in-placement value of $\$ 26,932,175$ in Table 8 . Estimated replacement costs are to the standard for the class of road that is required for the traffic volume; In-placement costs are for the dimensions of the road system as it currently exists. As an example, a low volume gravel road should have a platform width of 9 m and 550 mm of granular base, whereas the average gravel road platform in Strong is 7.5 m and there does not appear to be 450 mm of gravel base on the roads.

### 7.2 System Adequacy

The system adequacy is a measure of that portion of the system that is not categorized as a need in the "NOW" time period. The total road system adequacy is calculated as follows:

$$
\text { System Adequacy }=\frac{\text { Total System }(\mathrm{km})-\text { NOW Deficiencies }(\mathrm{km})}{\text { Total System }(\mathrm{km})} \times 100
$$

The System adequacy calculation provides a report card on the adequacy or appropriateness of the road programming since the last roads needs study. A decrease in the system adequacy reflects inadequate funding or inappropriate pavement management strategy.

As such, measuring and reviewing the trend in the system adequacy calculations over time is one of the most effective measures of the performance of the overall roads program.

The Township of Strong currently has a road system adequacy of $48.18 \%$. From a road system of 151.01 km (unadjusted for boundary roads), 78.25 km are rated as deficient in the 'NOW' time period (with $>50$ aadt). The traditional target adequacy for upper tier road systems (Regions and Counties) is $75 \%$ and a lower tier's target adequacy is $60 \%$. Based on these former MTO targets that were in effect when the municipal grant system was in place, the minimum target adequacy for Township of Strong, should be $60 \%$ as a minimum. The minimum target adequacies were established by MTO to reflect the nature and purpose of the road system.

## 8. Recommended Program Funding Levels

Recommended program funding level calculations are based on the length or number of the infrastructure types within the database and average widths of same as also determined from the database.

### 8.1 Capital Replacement -Roads

Recommended funding for the road system would include sufficient capital expenditures that would allow the replacement of infrastructure as it meets its design life.

For example, a typical road structure is expected to last approximately 50 years before it has to be reconstructed/replaced. If the lifespan is 50 years, then $2 \%$ of the replacement cost should be the annual contribution to a capital reserve to ensure that it can be reconstructed in that time frame. From a slightly different perspective, the annual capital program should be reflective of the lifespan of the element being considered.

Based on the foregoing, and the data shown in Section 6 of this report, the estimated minimum annual capital program for roads related infrastructure should be $\$ 538,600$ per year for Township of Strong to just maintain the current system adequacy. This estimate does not include bridges, culverts, sidewalks or street lighting.

The recommendation provided is based upon the replacement value of the Township of Strong road system, including adjustment factors, over a 50 year life cycle. Perhaps a simpler explanation would be an analogy to a car. A car is purchased and payments are made throughout the life of the car, which equates to the annual contribution. Throughout that life of the car, maintenance is required such as oil changes, brake and strut replacements and perhaps painting. This would parallel the need on a road to crack seal and overlay during the life cycle. These activities can extend the useful life of the pavement, thereby reducing lifecycle costs.

The calculations provided in this report are based on the total size of the road system. This should represent an opportunity to develop a financial plan to increase the capital and resurfacing budgets in conjunction with longer term program development.

### 8.2 Hot Mix Resurfacing (major maintenance)

Both roads and bridges require major maintenance activities throughout their life in order to reach those design life spans. Roads require resurfacing and bridges require waterproofing to be replaced and/or bridge deck rehabilitations at the correct interval. Some municipalities include these activities in the operating budget whereas others include them in the capital budget due to the dollar value.

The time interval between hot mix resurfacing cycles is dependant upon traffic loading and more particularly truck loading. Roads with higher percentage truck traffic have a shorter anticipated lifespan than local residential roads. Studies have shown that the optimal timing for a hot mix overlay on a road is between 12 and 20 years, depending on the road type. MTO 400 series roads would tend toward the 12 year cycle, with lower volume county Roads tending toward the 20 year replacement cycle.

Most municipalities resurface their local residential roads over a longer timeframe than they would an arterial road and a 25 year cycle for that activity is more typical. However, with the deferral of resurfacing past the ideal time interval comes risk. At 25 years the pavement surface may require additional rehabilitative effort beyond resurfacing. As such, the optimal budget calculation will focus on the 17 year interval for hot mix roads.

Given the foregoing, and the information with respect to surface type contained in Table 6, the funding for the annual resurfacing program size should be $\$ 26,300$ per year to maintain the system at its current adequacy level. This estimate is for the major resurfacing work only and does not include any estimated costs for other pavement preservation activities or programs. Given the short length of roads with a hot mix asphalt surface type in the Township of Strong, it would not be expected that this expenditure would occur annually; this dollar value represents the average annual contribution to the program.

### 8.3 Structure Deck Waterproofing and Rehabilitation (major maintenance)

Similar in nature to the hot mix resurfacing program, generally bridge decks require the waterproofing and pavement to be renewed at regular intervals. Bridge deck rehabilitations would generally be required at every other resurfacing cycle.

Figure 17 Bridge Deck Maintenance


### 8.4 Surface Treated Roads

On average, the life expectancy of a single surface treatment is 7 years. Following the life cycle replacement scenarios in the other asset areas then, the surface treated road network should be completely re-surfaced every 7 years-approximately $14 \%$ of the network per year.

At a unit cost of $\$ 2.50$ per square metre, including padding, the annual program size should be $\$ 95,900$ exclusive of hot mix asphalt padding and other preparatory work. This amount also does not include any surface treatment that would be applied as part of the gravel road conversion program.

### 8.5 Gravel Surface Roads

The standard practice for gravel road maintenance was that approximately 75 mm of gravel be placed on each road section every three years when MTO was providing maintenance subsidy.

Since the conditional grant system was discontinued, a large number of municipalities have reduced the amount of gravel that has been placed on gravel roads to the point where they are a major maintenance problem, particularly in the latter part of the winter and early spring. If the granular base is not replenished the road structure will disappear through normal usage and the remaining gravel generally becomes contaminated with other materials such as the native soil.

The Township of Strong has 105.47 kilometres of gravel surfaced roads as per Table 3 of this report. At a 3 year cycle, and using the municipality's benchmark costing, the annual gravel resurfacing program size should be $\$ 681,400$ per year. This estimate does not include costs for re-grading, dust control or ditching. This amount does not include any costs for a gravel road conversion program.

## 9. Pavement Management Systems and Strategies

The American Association of State Highway and Transportation Officials (AASHTO) defines asset management as "... a strategic approach to managing transportation infrastructure. It focuses on business processes for resource allocation and utilization with the objective of better decision-making based upon quality information and well-defined objectives."

The document entitled Managing Public Infrastructure Assets 2001 prepared by AMSA, AMWA, WEF, AWWA, defines asset management as "managing infrastructure assets to minimize the total cost of owning and operating them, while continuously delivering the service levels customers' desire, at an acceptable level of risk."

The absolute minimum objective of any pavement management strategy should be to ensure that the overall system adequacy does not decrease over time.

### 9.1 Overview of Pavement Management Systems (PMS)

Generally for a municipality, the road related infrastructure represents their largest asset or asset group. Efficient and effective management of the road system involves complex decision-making processes.

Collecting, maintaining, and analyzing, pavement condition data are the objectives of a PMS in order to maximize the performance of the municipal road network.

In practice still today, a large amount of the decision-making with respect to the maintenance of the road system occurs at the road supervisor level, based on their detailed knowledge of the roads system. Funding levels rarely match the demands.

The following table identifies how a pavement management system benefits the many potential user groups and perspectives.

Table 12 Benefits of a Pavement Management System

|  | Political | Programming | Budgeting <br> \& Financial | Engineering |
| :---: | :---: | :---: | :---: | :---: |
| System composition | - |  |  | - |
| Detailed Physical inventory |  |  |  | - |
| Overall System Adequacy | - |  |  | - |
| Condition Ratings |  | - |  | - |
| Rehabilitation options/costs |  | - | - | - |
| Budget Limitation Implications | - | - | - | - |
| Strategy | - | - |  | - |
| Project Coordination/ utilities |  | - |  | - |
| Priorities | - | - |  | - |
| Deterioration prediction | - | - | - | - |
| Managing Cash Flow |  |  | - | - |
| Fiscal Policy development | - | - | - | - |

The PMS is another tool in the toolbox to assist municipal Council's and staff in decision-making with the funding available. A PMS is a valuable decision-making tool at a number of levels in an organization including staff from technical, management, financial areas and politicians, who are ultimately responsible for the continued performance of the road system.

A PMS is important in providing analysis of strategies or funding levels and projecting the long term effect on the road system. The PMS provides the means to develop effective pavement management strategies for any agency.

### 9.2 Hot Mix Roads Pavement Management Strategies

One of the difficulties that road agencies encounter is the parochial nature of direction that can be provided. That direction is often counter to effective pavement management decision making.

There is a strong tendency to adopt a 'worst first' approach to project selection and unless the entire program is adequately funded, then the 'worst first' approach will lead to a further deterioration of the adequacy of the road system. Given the information with respect to system adequacies and effect programming may have on the system adequacy, the 'worst first' approach and its long term consequences should be carefully considered/reconsidered by any municipality before acting on it.

Of course there are other drivers in capital programming decision-making which are unavoidable, such as development demands. However, with a number of these other influencing factors, there should also be an alternate funding source, rather than the roads capital reserve.

As indicated earlier in this report, the minimum objective of any pavement management strategy should be to ensure that the overall system adequacy does not decrease over time. Given that most road agencies are inadequately funded, the majority of the discussion in the hot mix roads pavement management strategies section will focus on a road system with less than optimal funding.

### 9.3 Hot Mix Roads Pavement Management with Limited Resources

Once again, bearing in mind the minimum objective of maintaining system adequacy, if the overall roads program is not sufficiently funded, then the available funds should be expended on the maintaining the adequacy of the system. More simply stated - 'right size' the hot mix resurfacing program.

If the funding for the hot mix resurfacing program (or rehabilitation projects) is inadequate, then by default some of the resurfacing candidates will become reconstruction projects at 3 to 4 times the cost for a rural road; up to 7 times as much for an urban section. Therefore, it is critical that hot mix resurfacing occur in the optimal timeline or there will be deterioration in the overall system adequacy and with that, increased long term costs.

Deferral of a road project that is already categorized as a 'NOW' need, will not result in further deterioration of a road system's adequacy, however there will be increased maintenance costs for the road section and potentially more public complaint. Deferral of a hot mix resurfacing project will result in major cost implications for the road agency and may reduce the overall system adequacy, increased public concern and maintenance costs. Figure 18 from an APWA publication provides a graphical representation of the foregoing discussion.

A hot mix overlay at the optimal point in the deterioration curve is the most cost effective use of available funding. Adequate funding should be provided for the hot mix resurfacing program in order that the annual program is appropriately sized to ensure the continued adequacy of the existing road system and to prevent further deterioration.

Figure 18 Pavement Condition versus Rehabilitation Cost

PAVEMENT LIFE CYCLE: MAINTENANCE COST VS. CONDITION


Adapted From: American Public Works Association, 1983.
The Hole Story: Facts and Fallacies of Potholes

### 9.4 Pavement Preservation

Given the foregoing discussion, and optimizing the hot mix resurfacing program in order to preserve the system adequacy, the hot mix pavement strategy should also review opportunities for other pavement preservation activities.

Extending the life of an existing pavement further optimizes the usage of available funding. Other strategies that should be considered for integration into road programming include surface treating, microsurfacing, crack sealing, rejuvenators and other surface seals and treatments.

### 9.5 Project Prioritization

In a perfect world, with full funding, projects should be undertaken in order of priority, by program. The highest priority is to ensure that the hot mix resurfacing program is adequately funded. If funding is limited, then resurfacing should be prioritized over other programs such as construction. The world isn't perfect and the complexity of projects continues to increase as other agencies and utilities utilize the road allowance.

Projects should generally be undertaken in order of priority ranking by program; however the scoring system utilized in the PMS only rates/ranks more tangible criteria that exist in the database. There may be
other criteria that are specific to a municipality that are less tangible, but are important considerations in project prioritization. For example, a municipality may want to advance projects that also include bike lanes ahead of those roads that do not have, or will not have bike lanes.

The Roads Needs Study provides ratings that deal strictly with the condition of the roads and those indications have to be metered with needs that may exist for other utilities. For example, a road that is rated as a resurfacing candidate may have deficient sewers and watermains. The implication is that a significant percentage of the road would be excavated as those utilities are replaced. It would be appropriate then to re-rate the road as a reconstruction project.

The condition of other infrastructure within the road allowance may be the key element in the prioritization. For example, a road rated as a reconstruct project may have a relatively low priority rating but a trunk watermain in the street may require immediate replacement. It would be pragmatic then to advance that road re-construction project ahead of other road projects.

Frequently, a higher priority project may be undertaken adjacent to a much lower priority project that may not be scheduled to occur for years based on its own priority rating. If the lower priority project were to be advanced as a stand-alone project the unit costs may tend to make it disproportionately expensive due to the small quantities and location. Those circumstances may present an opportunity to advance the lower priority project to capture economies of scale that may not exist otherwise.

To summarize then, road projects should generally be undertaken in order of priority, however, in developing the capital program, other factors should also be taken into consideration such as;

- Other ranking criteria that may be specific to the municipality
- The condition of other infrastructure within the road
- Other infrastructure replacements may have a higher priority
- Realize opportunities of proximity and bulk purchasing.


### 9.6 Surface Treatment Roads Management Strategy

The management of the surface treated roads should be relatively straight forward. An annual schedule based on a 7 year cycle should be established and reasonably adhered to. Each spring, prior to the annual program being undertaken, the roads should be reviewed and program adjusted to reflect roads that are better or worse than anticipated.

Similar to the discussion with respect to the hot mix resurfacing program, a 'right-sized' surface treatment program would be key in optimizing funds and extending the life of the road system.

As the traffic grows on a surface treated road, it would be worthwhile to conduct a Net Present Value analysis of the surface treated surface versus a hot mix asphalt surface.

### 9.7 Gravel Roads Management Strategy

Proper maintenance of a gravel road surface is deceptively expensive. Once the costs of gravel, dust control and grading are considered, often the cost per kilometre of gravel road maintenance is increased
to the point where it is greater than the cost to maintain a hard-topped road section. At that point it may be cost effective to convert/upgrade the gravel road to a surface treated road.

Studies from various agencies, both in Canada and the United States, have shown, that dependant upon local unit costs for materials and machinery, conversion of a structurally sound gravel road to a surface treated road can be a cost effective strategy for roads with traffic volumes as low as 100 AADT. Net Present Value and Payback period analysis of this option can be developed that are specific to local material costs.

Once the above noted analysis has been completed and proves viable, candidate project selection could include roads with the following characteristics:

- adequate existing granular base structure (typically a minimum of 450 mm of material in southern Ontario, 550 in northern Ontario; 150 mm of Granular A and 300 of Granular B; 400 mm of Granular B in Northern Ontario)
- adequate drainage;
- high maintenance costs (frequent complaints and calls);
- isolation from other gravel roads (high deadheading costs);
- sections that would provide continuity in a hard top network;
- proximity to work that is being done in other programs, for example asphalt millings to supplement gravel program.

Typically, a large percentage of gravels roads are indicated as 'NOW' needs in most road inventories. It should be noted that a 'NOW' rating can be triggered by either sub-standard structure or geometric deficiency such as the surface width.

Conversion of a gravel road to a surface treated road may not necessarily raise the road out of the 'NOW' needs category as the inherent geometric and surface width deficiencies would remain. However, over time, converting gravel surfaced roads to surface treated roads will generally reduce overall operating costs.

Benefits to converting a gravel road include:

- customer satisfaction
- reduced maintenance costs for routine maintenance
- reduced maintenance costs for winter maintenance
- reduced complaints.

Another option that the municipality may wish to consider is providing additional funding to add additional gravel to those roads that are not structurally adequate with the intention of surface treating the road in a subsequent year.

### 9.8 Subdivision Roads/Development Management Strategy

As development occurs, new roads are added to the road network, and thus presents a future financial liability for the municipality.

The capital and operating budget should be adjusted annually to reflect the increased road network. Some municipalities deal with this issue as a system size adjustment, or a base adjustment, over and above any inflationary increases that may be required to manage the road system. For example, if the system size grows by two per cent then the related roads budget items should increase by that same amount over and above all other increases, in order that the same service level is maintained.

### 9.9 Annual Budget Adjustment

Generally budgets are adjusted on an annual basis by most municipalities and the average Consumer Price Index is usually the targeted amount. Adopting this practice for public works and particularly road infrastructures ensures a continual downward spiral in overall condition of the road system and service levels. Given the increasing litigious nature of our society, decreased and/or inadequate funding increases the exposure to risk for the municipality.

Given the disproportionate increases that have occurred in fuel, asphalt and salt over the last few years, consideration should be given annually to increasing the road funding over and above the CPI in order that service levels may be maintained

## 10. Township of Strong Pavement Management Strategy Summary

If the capital budget remains at its current level, the Township of Strong will have significant challenges in maintaining the adequacy of its current road system, as evidenced by the difference in current funding versus the recommendations for program funding levels.

The following asset management strategy is recommended:

- Revise the programming to prioritize preservation and life extensions activities for roads and structures. These activities would include
- Deferring the reconstruction program in favour of preservation and life extension activities
- Optimizing hot mix overlay program
- Optimizing the surface treatment program
- Ensuring structure inspections are completed every two years
- Defer replacement of structures that are structurally sound that have a geometric need; improve the signage on those structures in the interim.
- Prioritize structure major maintenance activities such as rehabilitations and resurfacing
- At the time of structure rehabilitations, review the opportunity for conversion to semiintegral type structures
- Review the opportunities for a gravel road conversion program
- If additional funding becomes available, such as through a grant program, it should be directed to a reconstruction project.
NOW Construction

| Priority Ranking | Section No. | Road <br> Name | From/ To | AADT | Length (km) | Improv. Time | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 33 | 00105 | STRONG JOLY ROAD | FOREST LAKE ROAD <br> 175M NORTH OF FOREST LAKE ROAD | 300 | 0.18 | NOW | REC | 59,998 |
| 34 | 00130 | FOREST LAKE ROAD | STRONG JOLY ROAD <br> INLET CREEK ROAD | 450 | 0.99 | NOW | REC | 511,985 |
| 31 | 00140 | FOREST LAKE ROAD | INLET CREEK ROAD LAKESHORE DRIVE | 450 | 1.29 | NOW | REC | 667,131 |
| 42 | 00160 | LAKESHORE DRIVE | 150M SOUTH OF BERNARD CRESCENT PEVENSEY ROAD | 425 | 1.00 | NOW | REC | 488,877 |
| 21 | 00170 | PEVENSEY ROAD | SUNNY BEACH ROAD <br> 40M WEST OF INLET CREEK ROAD | 200 | 0.66 | NOW | REC | 219,994 |
| 38 | 00230 | PEVENSEY ROAD | $\begin{aligned} & \text { MUSKOKA ROAD } \\ & \text { 2.8km East } \end{aligned}$ | 190 | 2.80 | NOW | REC | 838,824 |
| 43 | 00250 | SOUTH LAKE BERNARD ROAD | HIGHWAY 11 <br> MUSKOKA ROAD | 750 | 1.27 | NOW | BS | 376,056 |
| 39 | 00270 | RODEO ROAD | SOUTH HORN LAKE ROAD NORTH HORN LAKE ROAD | 150 | 1.53 | NOW | REC | 458,357 |
| 34 | 00320 | RODEO ROAD | NORTH HORN LAKE ROAD BLACK CREEK ROAD | 150 | 2.06 | NOW | REC | 617,135 |
| 24 | 00340 | BLACK CREEK ROAD | RODEO ROAD HIGHWAY 11 | 50 | 1.24 | NOW | REC | 312,200 |
| 25 | 00350 | RODEO ROAD | BLACK CREEK ROAD <br> BROOKSIDE ROAD | 150 | 2.37 | NOW | REC | 596,706 |
| 23 | 00360 | BROOKSIDE ROAD | $\begin{aligned} & \text { CHAPMAN STRONG ROAD } \\ & \text { RODEO ROAD } \end{aligned}$ | 150 | 2.03 | NOW | BS | 304,273 |
| 42 | 00370 | CHAPMAN STRONG ROAD | SOUTH END BROOKSIDE ROAD | 120 | 1.06 | NOW | REC | 317,555 |
| 34 | 00380 | CHAPMAN STRONG ROAD | BROOKSIDE ROAD BLOOMFIELD ROAD | 100 | 2.01 | NOW | REC | 602,156 |
| 35 | 00390 | BLOOMFIELD ROAD | CHAPMAN STRONG ROAD <br> HIGHWAY 124 | 100 | 1.73 | NOW | REC | 518,273 |
| 20 | 00470 | HORNIBROOK ROAD | ADAMS ROAD PARKES LANE | 100 | 2.03 | NOW | BS | 266,939 |

NOW Construction

| Priority Ranking | Section <br> No. | Road Name | From/ To | AADT | Length (km) | Improv. Time | Improv. Type | Improv. Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 00480 | PARKES LANE | WEST END HORNIBROOK ROAD | 60 | 3.05 | NOW | REC | 913,719 |
| 21 | 00490 | HORNIBROOK ROAD | PARKES LANE <br> MACHAR STRONG BOUNDARY ROAD WEST | 200 | 2.05 | NOW | BS | 366,951 |
| 25 | 00520 | HILL VALLEY ROAD | HORNIBROOK ROAD UPLANDS ROAD | 100 | 1.65 | NOW | BS | 365,685 |
| 17 | 00530 | HILL VALLEY ROAD | UPLANDS ROAD ALBERT STREET | 100 | 2.42 | NOW | BS | 536,337 |
| 22 | 00430 | COTTRALL ROAD | HIGHWAY 124 <br> ADAMS ROAD | 75 | 1.55 | NOW | REC | 2,015,223 |
| 27 | 00050 | MACHAR STRONG BOUNDARY ROAD WEST | 1.15KM WEST OF ALBERT STREET ALBERT STREET | 300 | 1.15 | NOW | BS | 203,739 |
| 29 | 00070 | MACHAR STRONG BOUNDARY ROAD WEST | ALBERT STREET TOWER ROAD | 300 | 2.04 | NOW | BS | 415,764 |
| 28 | 00560 | TOWER ROAD | MACHAR STRONG BOUNDARY ROAD WEST HILL VALLEY ROAD | 200 | 1.99 | NOW | BS | 367,173 |
| 29 | 00570 | TOWER ROAD | HILL VALLEY ROAD HIGH STREET | 200 | 2.08 | NOW | BS | 383,779 |
| 24 | 00580 | HIGH STREET | TOWER ROAD HIGHWAY 11 | 100 | 0.39 | NOW | REC | 147,546 |
| 12 | 00620 | CHERYL CRESCENT | FOREST LAKE ROAD <br> FOREST LAKE ROAD | 100 | 0.56 | NOW | BS | 146,148 |
| 19 | 00630 | BERNARD CRESCENT | FOREST LAKE ROAD LAKESHORE DRIVE | 100 | 0.68 | NOW | REC | 257,260 |
| 17 | 00640 | EVERGREEN LANE | BERNARD CRESCENT <br> EAST END CULDESAC | 80 | 0.09 | NOW | REC | 34,049 |
| 27 | 00650 | HOMESTEAD LANE | BERNARD CRESCENT EAST END | 50 | 0.09 | NOW | REC | 29,971 |
| 30 | 00660 | BUCKO MCDONALD DRIVE | FOREST LAKE ROAD <br> SOUTH/EAST END TURNAROUND | 90 | 0.39 | NOW | REC | 129,876 |
| 31 | 00680 | MALYON DRIVE | FOREST LAKE ROAD SOUTH END | 130 | 0.60 | NOW | REC | 226,994 |
| 32 | 00720 | SUNNY BEACH ROAD | $\underset{\text { LAKE }}{\text { LAKESHORE DRIVE }}$ | 50 | 0.15 | NOW | REC | 41,635 |

NOW Construction

| Priority Ranking | $\begin{aligned} & \text { Section } \\ & \text { No. } \end{aligned}$ | Road Name | $\begin{gathered} \text { From/ } \\ \text { To } \\ \hline \end{gathered}$ | AADT | Length (km) | Improv. Time | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 27 | 00740 | ELIZABETH STREET | LAKESHORE DRIVE WEST END | 170 | 0.40 | NOW | REC | 454,228 |
| 25 | 00710 | LAKESHORE DRIVE | PEVENSEY ROAD <br> 170M WEST OF PEVENESY ROAD | 225 | 0.17 | NOW | REC | 47,187 |
| 27 | 00790 | SOUTH LAKE BERNARD ROAD | FLANIGAN TRAIL PIPES O PAN LANE | 75 | 3.21 | NOW | REC | 961,651 |
| 33 | 00830 | SOUTH LAKE BERNARD ROAD | HIGH ROCK DRIVE MUSKOKA ROAD | 390 | 1.53 | NOW | BS | 335,647 |
| 29 | 00840 | CRESCENT ROAD | SOUTH LAKE BERNARD ROAD <br> WEST END | 50 | 0.41 | NOW | REC | 155,113 |
| 27 | 00850 | HIGH ROCK DRIVE | SOUTH LAKE BERNARD ROAD CRESCENT ROAD | 390 | 0.42 | NOW | BS | 75,180 |
| 26 | 00860 | HIGH ROCK DRIVE | CRESCENT ROAD BIRCH LANE | 390 | 1.27 | NOW | BS | 227,331 |
| 33 | 00940 | HIGH ROCK DRIVE | 120M NORTH OF BRIDGE <br> TURTLE ROCK LANE | 390 | 2.22 | NOW | BS | 401,458 |
| 30 | 00960 | HIGH ROCK DRIVE | TURTLE ROCK LANE LAYOLOMI DRIVE | 390 | 0.89 | NOW | BS | 173,046 |
| 30 | 00980 | HIGH ROCK DRIVE | LAYOLOMI DRIVE ALBERT STREET | 390 | 1.96 | NOW | BS | 361,637 |
| 29 | 00990 | ALBERT STREET | HIGH ROCK DRIVE 200M SOUTH OF SUNSET DRIVE | 390 | 0.49 | NOW | REC | 170,937 |
| 39 | 01040 | MUSKOKA ROAD | SOUTH LAKE BERNARD ROAD SOUTH LAKE BERNARD ROAD | 540 | 0.91 | NOW | BS | 269,458 |
| 27 | 01060 | MUSKOKA ROAD | SOUTH LAKE BERNARD ROAD ROBINS ROAD | 150 | 1.77 | NOW | BS | 265,302 |
| 17 | 01080 | MUSKOKA ROAD | ROBINS ROAD <br> VALLEY VIEW ROAD | 150 | 2.58 | NOW | BS | 339,262 |
| 42 | 00900 | BIRCH LANE | ASPEN LANE <br> WEST END | 100 | 0.72 | NOW | REC | 158,331 |
| 50 | 01130 | SUNNY RIDGE ROAD | HIGHWAY 11 HIGH ROCK DRIVE | 400 | 1.50 | NOW | REC | 872,553 |
| 25 | 01150 | BUCK HAVEN ROAD | SUNNY RIDGE ROAD HIGHWAY 11 | 200 | 0.41 | NOW | BS | 71,885 |

1-5 Construction

| Priority Ranking | Section No. | Road <br> Name | $\begin{gathered} \text { From/ } \\ \text { To } \\ \hline \end{gathered}$ | AADT | Length <br> (km) | Improv. Time | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 26 | 01360 | FARM VIEW ROAD | PEVENSEY ROAD <br> WEST END | 50 | 2.45 | 1-5 | REC | 733,971 |
| 20 | 01270 | HILLCREST STREET | OAKRIDGE DRIVE <br> ALBERT STREET | 50 | 0.14 | 1-5 | REC | 52,965 |
| 30 | 01010 | ALBERT STREET | SUNSET DRIVE <br> MAIN STREET | 390 | 0.17 | 1-5 | RSS | 250,621 |
| 31 | 01120 | MAPLE SUGAR LANE | HIGHWAY 11 NORTH END | 150 | 0.12 | 1-5 | REC | 156,017 |
| 23 | 00880 | BIRCH LANE | HIGH ROCK DRIVE THE POINT TRAIL | 120 | 0.33 | 1-5 | REC | 98,861 |
| 22 | 00800 | SOUTH LAKE BERNARD ROAD | PIPES O PAN LANE 600M WEST OF PIPES O PAN LANE | 200 | 0.60 | 1-5 | REC | 210,671 |
| 18 | 00750 | SOUTH LAKE BERNARD ROAD | LAKESHORE DRIVE GILCHRIST TRAIL | 50 | 2.93 | 1-5 | REC | 737,700 |
| 21 | 00780 | SOUTH LAKE BERNARD ROAD | GILCHRIST TRAIL <br> FLANIGAN TRAIL | 50 | 1.83 | 1-5 | REC | 460,747 |
| 17 | 00440 | ADAMS ROAD | COTTRELLS ROAD <br> HORNIBROOK ROAD | 125 | 0.81 | 1-5 | REC | 203,937 |
| 21 | 00310 | NORTH HORN LAKE ROAD | MAGNETAWAN BOUNDARY RODEO ROAD | 60 | 1.64 | 1-5 | REC | 2,537,091 |
| 20 | 00040 | MACHAR STRONG BOUNDARY ROAD WEST | OLD MUSKOKA ROAD SOUTH <br> 1.15KM WEST OF ALBERT ROAD | 300 | 1.71 | 1-5 | BS | 299,812 |
| 28 | 00500 | CHEER LAKE ROAD | MACHAR STRONG BOUNDARY ROAD WEST SOUTH END | 100 | 2.25 | 1-5 | BS | 337,249 |
| 23 | 00240 | MUSKOKA ROAD | PEVENSEY ROAD <br> SOUTH LAKE BERNARD ROAD | 110 | 1.18 | 1-5 | REC | 353,504 |
| 30 | 00220 | PEVENSEY ROAD | HIGHWAY 11 <br> MUSKOKA ROAD | 190 | 1.18 | 1-5 | REC | 353,504 |
| Totals: | 1-5 | truction |  |  | 17.34 |  |  | 6,786,652 |

6-10 Construction

| Priority Ranking | Section No. | Road <br> Name | From/ To | AADT | Length (km) | Improv. Time | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 19 | 00100 | STRONG JOLY ROAD | PEACOCK ROAD <br> 175 M NORTH OF FOREST LAKE ROAD | 300 | 1.86 | 6-10 | REC | 591,102 |
| 25 | 00200 | INLET CREEK ROAD | PEVENSEY ROAD <br> FOREST LAKE ROAD | 100 | 1.66 | 6-10 | REC | 497,303 |
| 26 | 00260 | NORTH HORN LAKE ROAD | HIGHWAY 11 RODEO ROAD | 300 | 1.26 | 6-10 | REC | 476,454 |
| 13 | 00280 | SOUTH HORN LAKE ROAD | MAGNETEWAN BOUNDARY RODEO ROAD | 75 | 1.16 | 6-10 | REC | 347,513 |
| 17 | 00290 | SOUTH HORN LAKE ROAD | RODEO ROAD <br> ARMOUR BOUNDARY | 75 | 0.21 | 6-10 | REC | 62,912 |
| 20 | 00300 | RODEO ROAD | NORTH HORN LAKE ROAD NORTH HORN LAKE ROAD | 150 | 0.86 | 6-10 | BS | 113,087 |
| 26 | 00550 | ALBERT STREET | HILL VALLEY ROAD <br> MACHAR STRONG BOUNDARY ROAD WEST | 100 | 2.04 | 6-10 | REC | 513,620 |
| 31 | 00400 | RODEO ROAD | BROOKSIDE ROAD HIGHWAY 124 | 150 | 2.05 | 6-10 | REC | 614,139 |
| 34 | 00600 | BASSO ROAD | EAST SIDE OF RAILWAY TRACKS EAST END | 100 | 1.26 | 6-10 | REC | 377,471 |
| 19 | 01000 | ALBERT STREET | 200M SOUTH OF SUNSET DRIVE SUNSET DRIVE | 390 | 0.20 | 6-10 | REC | 99,985 |
| 9 | 00920 | HIGH ROCK DRIVE | $\begin{aligned} & \text { BIRCH LANE } \\ & \text { 120M SOUTH OF BRIDGE } \end{aligned}$ | 390 | 0.78 | 6-10 | BS | 136,756 |
| 22 | 01090 | VALLEY VIEW ROAD | HIGHWAY 11 MUSKOKA ROAD | 50 | 0.72 | 6-10 | REC | 158,331 |
| 14 | 01110 | MUSKOKA ROAD | VALLEY VIEW ROAD HIGHWAY 11 | 150 | 1.14 | 6-10 | BS | 149,907 |
| 13 | 01250 | ALBERT STREET | 500M NORTH OF ADAMS ROAD 700M SOUTH OF HILL VALLEY ROAD | 200 | 0.85 | 6-10 | BS | 160,235 |
| 13 | 01170 | SUNSET DRIVE | HIGHWAY 11 <br> ALBERT STREET | 50 | 0.39 | 6-10 | REC | 147,546 |
| 13 | 01140 | GOLF LANE | SUNNY RIDGE ROAD SOUTH END | 50 | 0.18 | 6-10 | BS | 46,976 |
| 17 | 01370 | KENTS MILL ROAD | PEVENSEY ROAD JOLY BOUNDARY | 50 | 0.84 | 6-10 | REC | 184,720 |



| Priority <br> Ranking | Section <br> No. | Road <br> Name | From/ |
| :--- | :--- | :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| 28 | 00690 | To |  |


| Priority Ranking | Section <br> No. | Road <br> Name | From/ | AADT | $\underset{(\mathrm{km})}{\text { Length }}$ | Improv. Time | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 28 | 00700 | FOREST LAKE ROAD | BUCKO MCDONALD DRIVE LAKESHORE DRIVE | 1,500 | 0.39 | 6-10 | R1 | 51,606 |
| Totals: | 6-10 |  |  |  | 0.39 |  |  | 51,606 |

6-10 Maintenance

| Priority Ranking | Section No. | Road <br> Name | From/ To | AADT | Length (km) | Improv. Time | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16 | 00730 | LAKESHORE DRIVE | 170M WEST OF PEVENESY ROAD SOUTH LAKE BERNARD ROAD | 225 | 0.62 | 6-10 | SD | 0 |
| 11 | 00030 | MACHAR STRONG BOUNDARY ROAD WEST | MORRIS LANE OLD MUSKOKA ROAD SOUTH | 300 | 3.07 | 6-10 | SD | 0 |
| 17 | 00150 | LAKESHORE DRIVE | LAKESHORE DRIVE <br> 150M SOUTH OF BERNARD CRESCENT | 425 | 0.43 | 6-10 | SD | 0 |
| 14 | 01200 | ADAMS ROAD | 900M WEST OF ALBERT STREET ALBERT STREET | 200 | 0.90 | 6-10 | SD | 0 |
| Totals: | 6-10 | Maintenance |  |  | 5.02 |  |  | 0 |
| Grand Total: |  |  |  |  | 119.22 |  |  | 34,974,555 |

Appendix B Critical Deficiencies and Recommended Improvements Summary For Roads
Critical Deficiencies and Recommended Improvements

| Section <br> No. | Road <br> Name | $\begin{gathered} \text { From/ } \\ \text { To } \end{gathered}$ | Length $(k m)$ | AADT | $\begin{gathered} \text { Geo- } \\ \text { metrics } \end{gathered}$ | Surf. Type | Surf. Width | Capacity | Struct. Adeq. | Drainage | Improv. <br> Type | Improv <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00010 | MACHAR STRONG BOUNDARY ROAD | CHEER LAKE ROAD <br> PARK ROAD SOUTH | 1.63 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | SD | 0 |
| 00020 | MACHAR STRONG BOUNDARY ROAD | PARK ROAD SOUTH MORRIS LANE | 0.65 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | SD | 0 |
| 00030 | MACHAR STRONG BOUNDARY ROAD | MORRIS LANE <br> OLD MUSKOKA ROAD SOUTH | 3.07 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | SD | 0 |
| 00040 | MACHAR STRONG BOUNDARY ROAD | OLD MUSKOKA ROAD SOUTH <br> 1.15KM WEST OF ALBERT ROAD | 1.71 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | 6-10 | BS | 299,812 |
| 00050 | MACHAR STRONG BOUNDARY ROAD | 1.15KM WEST OF ALBERT STREET ALBERT STREET | 1.15 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 203,739 |
| 00070 | MACHAR STRONG BOUNDARY ROAD | ALBERT STREET <br> TOWER ROAD | 2.04 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 415,764 |
| 00080 | STRONG JOLY ROAD | AIRPORT ROAD <br> RAILWAY TRACKS/ MACHAR BOUNDARY | 1.01 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 00090 | STRONG JOLY ROAD | AIRPORT ROAD <br> PEACOCK ROAD | 0.99 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 00100 | STRONG JOLY ROAD | PEACOCK ROAD <br> 175 M NORTH OF FOREST LAKE ROAD | 1.86 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 591,102 |
| 00105 | STRONG JOLY ROAD | FOREST LAKE ROAD <br> 175M NORTH OF FOREST LAKE ROAD | 0.18 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 59,998 |
| 00110 | PEACOCK ROAD | STRONG JOLY ROAD HIGHWAY 11 | 0.62 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 00120 | BROOKS LANE | PEACOCK ROAD SOUTH END | 0.89 | 20 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | None | 0 |
| 00130 | FOREST LAKE ROAD | STRONG JOLY ROAD INLET CREEK ROAD | 0.99 | 450 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 511,985 |
| 00140 | FOREST LAKE ROAD | INLET CREEK ROAD LAKESHORE DRIVE | 1.29 | 450 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 667,131 |
| 00150 | LAKESHORE DRIVE | LAKESHORE DRIVE <br> 150M SOUTH OF BERNARD CRESCENT | 0.43 | 425 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | SD | 0 |
| 00160 | LAKESHORE DRIVE | 150M SOUTH OF BERNARD CRESCENT PEVENSEY ROAD | 1.00 | 425 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 488,877 |
| 00170 | PEVENSEY ROAD | SUNNY BEACH ROAD <br> 40M WEST OF INLET CREEK ROAD | 0.66 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | REC | 219,994 |
| 00180 | PEVENSEY ROAD | 40M WEST OF INLET CREEK ROAD INLET CREEK ROAD | 0.06 | 100 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | SD | 0 |


| Section <br> No. | Road <br> Name | $\begin{gathered} \text { From/ } \\ \text { To } \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (\mathrm{km}) \end{array}$ | AADT | $\begin{gathered} \text { Geo- } \\ \text { metrics } \end{gathered}$ | Surf. <br> Type | Surf. <br> Width | Capacity | Struct. Adeq. | Drainage | Improv. Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00190 | OLD RANCH ROAD | INLET CREEK ROAD JOLY BOUNDARY | 0.81 | 5 | NOW | ADEQ | NOW | ADEQ | NOW | NOW | None | 0 |
| 00200 | INLET CREEK ROAD | PEVENSEY ROAD <br> FOREST LAKE ROAD | 1.66 | 100 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 497,303 |
| 00210 | STERLING CREEK ROAD | HIGHWAY 11 <br> WEST END | 0.35 | 30 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | None | 0 |
| 00220 | PEVENSEY ROAD | HIGHWAY 11 <br> MUSKOKA ROAD | 1.18 | 190 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | REC | 353,504 |
| 00230 | PEVENSEY ROAD | MUSKOKA ROAD 2.8 km East | 2.80 | 190 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | NOW | REC | 838,824 |
| 00240 | MUSKOKA ROAD | PEVENSEY ROAD <br> SOUTH LAKE BERNARD ROAD | 1.18 | 110 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | REC | 353,504 |
| 00250 | SOUTH LAKE BERNARD ROAD | HIGHWAY 11 <br> MUSKOKA ROAD | 1.27 | 750 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | ADEQ | BS | 376,056 |
| 00260 | NORTH HORN LAKE ROAD | HIGHWAY 11 RODEO ROAD | 1.26 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 476,454 |
| 00270 | RODEO ROAD | SOUTH HORN LAKE ROAD <br> NORTH HORN LAKE ROAD | 1.53 | 150 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | NOW | REC | 458,357 |
| 00280 | SOUTH HORN LAKE ROAD | MAGNETEWAN BOUNDARY RODEO ROAD | 1.16 | 75 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 347,513 |
| 00290 | SOUTH HORN LAKE ROAD | RODEO ROAD <br> ARMOUR BOUNDARY | 0.21 | 75 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 62,912 |
| 00300 | RODEO ROAD | NORTH HORN LAKE ROAD <br> NORTH HORN LAKE ROAD | 0.86 | 150 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | BS | 113,087 |
| 00310 | NORTH HORN LAKE ROAD | MAGNETAWAN BOUNDARY RODEO ROAD | 1.64 | 60 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | REC | 2,537,091 |
| 00320 | RODEO ROAD | NORTH HORN LAKE ROAD BLACK CREEK ROAD | 2.06 | 150 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | REC | 617,135 |
| 00330 | BLACK CREEK ROAD | WEST END RODEO ROAD | 0.50 | 25 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | None | 0 |
| 00340 | BLACK CREEK ROAD | RODEO ROAD HIGHWAY 11 | 1.24 | 50 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | NOW | REC | 312,200 |
| 00350 | RODEO ROAD | BLACK CREEK ROAD BROOKSIDE ROAD | 2.37 | 150 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | NOW | REC | 596,706 |
| 00360 | BROOKSIDE ROAD | CHAPMAN STRONG ROAD RODEO ROAD | 2.03 | 150 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 304,273 |
| 00370 | CHAPMAN STRONG ROAD | SOUTH END BROOKSIDE ROAD | 1.06 | 120 | ADEQ | ADEQ | NOW | ADEQ | NOW | 1-5 | REC | 317,555 |
| 00380 | CHAPMAN STRONG ROAD | BROOKSIDE ROAD BLOOMFIELD ROAD | 2.01 | 100 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 602,156 |




| $\begin{aligned} & \text { Section } \\ & \text { No. } \\ & \hline \end{aligned}$ | Road <br> Name | From/ To | Length (km) | AADT | $\begin{gathered} \text { Geo- } \\ \text { metrics } \end{gathered}$ | $\begin{aligned} & \hline \text { Surf. } \\ & \text { Type } \\ & \hline \end{aligned}$ | Surf. Width | Capacity | Struct. Adeq. | Drainage | $\begin{gathered} \text { Improv. } \\ \text { Type } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { Improv. } \\ & \text { Cost (\$) } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00790 | SOUTH LAKE BERNARD ROAD | FLANIGAN TRAIL PIPES O PAN LANE | 3.21 | 75 | NOW | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | REC | 961,651 |
| 00800 | SOUTH LAKE BERNARD ROAD | PIPES O PAN LANE 600M WEST OF PIPES O PAN LANE | 0.60 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | 6-10 | REC | 210,671 |
| 00810 | SOUTH LAKE BERNARD ROAD | 600M WEST OF PIPES O PAN LANE CRESCENT ROAD | 0.49 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 00820 | SOUTH LAKE BERNARD ROAD | CRESCENT ROAD HIGH ROCK DRIVE | 0.46 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 00830 | SOUTH LAKE BERNARD ROAD | HIGH ROCK DRIVE MUSKOKA ROAD | 1.53 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 335,647 |
| 00840 | CRESCENT ROAD | SOUTH LAKE BERNARD ROAD WEST END | 0.41 | 50 | ADEQ | NOW | NOW | ADEQ | ADEQ | 1-5 | REC | 155,113 |
| 00850 | HIGH ROCK DRIVE | SOUTH LAKE BERNARD ROAD CRESCENT ROAD | 0.42 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | ADEQ | BS | 75,180 |
| 00860 | HIGH ROCK DRIVE | CRESCENT ROAD BIRCH LANE | 1.27 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | ADEQ | BS | 227,331 |
| 00870 | CRESCENT ROAD | HIGH ROCK DRIVE EAST END | 0.18 | 40 | ADEQ | NOW | NOW | ADEQ | ADEQ | 1-5 | None | 0 |
| 00880 | BIRCH LANE | HIGH ROCK DRIVE THE POINT TRAIL | 0.33 | 120 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | REC | 98,861 |
| 00890 | ASPEN LANE | BIRCH LANE SOUTH END | 0.21 | 40 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | None | 0 |
| 00900 | BIRCH LANE | ASPEN LANE WEST END | 0.72 | 100 | NOW | ADEQ | NOW | ADEQ | NOW | 1-5 | REC | 158,331 |
| 00910 | THE POINT TRAIL | BIRCH LANE <br> WEST END | 0.12 | 35 | NOW | ADEQ | ADEQ | ADEQ | NOW | 1-5 | None | 0 |
| 00920 | HIGH ROCK DRIVE | BIRCH LANE <br> 120M SOUTH OF BRIDGE | 0.78 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | ADEQ | BS | 136,756 |
| 00930 | HIGH ROCK DRIVE | 120M SOUTH OF BRIDGE <br> 120M NORTH OF BRIDGE | 0.20 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 00940 | HIGH ROCK DRIVE | 120M NORTH OF BRIDGE TURTLE ROCK LANE | 2.22 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 401,458 |
| 00950 | TURTLE ROCK LANE | HIGH ROCK DRIVE EAST END | 0.32 | 25 | NOW | ADEQ | ADEQ | ADEQ | NOW | 1-5 | None | 0 |
| 00960 | HIGH ROCK DRIVE | TURTLE ROCK LANE LAYOLOMI DRIVE | 0.89 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 173,046 |
| 00970 | LAYOLOMI DRIVE | HIGH ROCK DRIVE SOUTH END | 0.26 | 30 | ADEQ | NOW | ADEQ | ADEQ | NOW | 6-10 | None | 0 |
| 00980 | HIGH ROCK DRIVE | LAYOLOMI DRIVE <br> ALBERT STREET | 1.96 | 390 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 361,637 |



| Section <br> No. | Road Name | $\begin{gathered} \text { From/ } \\ \text { To } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (k m) \\ \hline \end{array}$ | AADT |  |  |  |  |  |  | Improv. <br> Type | Improv. <br> Cost (\$) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{gathered} \text { Geo- } \\ \text { metrics } \end{gathered}$ | $\begin{aligned} & \hline \text { Surf. } \\ & \text { Type } \end{aligned}$ | Surf. Width | Capacity | Struct. <br> Adeq. | Drainage |  |  |
| 01190 | ADAMS ROAD | COTTRELLS ROAD <br> 900M WEST OF ALBERT STREET | 2.51 | 200 | NOW | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 949,126 |
| 01200 | ADAMS ROAD | 900M WEST OF ALBERT STREET ALBERT STREET | 0.90 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | SD | 0 |
| 01210 | ALBERT STREET | HIGHWAY 11 DUNBAR STREET | 0.31 | 500 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 157,661 |
| 01220 | ALBERT STREET | DUNBAR STREET OAKRIDGE DRIVE | 0.38 | 500 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 115,601 |
| 01230 | ALBERT STREET | OAKRIDGE DRIVE ADAMS ROAD | 0.49 | 500 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 234,354 |
| 01240 | ALBERT STREET | $\begin{aligned} & \text { ADAMS ROAD } \\ & 500 \mathrm{M} \text { NORTH OF ADAMS ROAD } \end{aligned}$ | 0.50 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 239,136 |
| 01250 | ALBERT STREET | 500M NORTH OF ADAMS ROAD 700M SOUTH OF HILL VALLEY ROAD | 0.85 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | BS | 160,235 |
| 01255 | ALBERT STREET | 700M SOUTH OF HILL VALLEY ROAD HILL VALLEY ROAD | 0.70 | 200 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 131,958 |
| 01260 | OAKRIDGE DRIVE | ALBERT STREET ADAMS ROAD | 0.79 | 150 | ADEQ | NOW | ADEQ | ADEQ | ADEQ | 1-5 | REC | 352,566 |
| 01270 | HILLCREST STREET | OAKRIDGE DRIVE <br> ALBERT STREET | 0.14 | 50 | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | 1-5 | REC | 52,965 |
| 01280 | HILL VALLEY ROAD | ALBERT STREET <br> TOWER ROAD | 2.04 | 100 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | NOW | REC | 611,143 |
| 01290 | HILL VALLEY ROAD | TOWER ROAD EAST END | 1.43 | 30 | NOW | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | None | 0 |
| 01300 | PEVENSEY ROAD | 40M WEST OF INLET CREEK ROAD 620M SOUTH OF INLET CREEK ROAD | 0.69 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | BS | 139,980 |
| 01310 | PEVENSEY ROAD | 620M SOUTH OF INLET CREEK ROAD TRUDGEONS ROAD | 1.40 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 01315 | TRUDGEONS ROAD | PEVENSEY ROAD <br> EAST END | 1.30 | 5 | NOW | ADEQ | NOW | ADEQ | NOW | NOW | None | 0 |
| 01320 | PEVENSEY ROAD | TRUDGEONS ROAD <br> MAPLE VALLEY ROAD | 1.98 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 01330 | MAPLE VALLEY ROAD | PEVENSEY ROAD <br> WEST END | 0.90 | 10 | NOW | ADEQ | ADEQ | ADEQ | NOW | NOW | None | 0 |
| 01340 | PEVENSEY ROAD | MAPLE VALLEY ROAD KENTS MILL ROAD | 2.17 | 300 | ADEQ | ADEQ | NOW | ADEQ | NOW | 6-10 | REC | 820,560 |
| 01350 | GIBBONS ROAD | PEVENSEY ROAD JOLY BOUNDARY | 0.80 | 50 | NOW | ADEQ | NOW | ADEQ | ADEQ | 1-5 | REC | 239,664 |
| 01360 | FARM VIEW ROAD | PEVENSEY ROAD WEST END | 2.45 | 50 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 1-5 | REC | 733,971 |


|  |  |  |  |  | Critical Deficiency |  |  |  |  |  | Improv. <br> Type | $\begin{aligned} & \text { Improv. } \\ & \text { Cost (\$) } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section No. | Road <br> Name | $\begin{gathered} \text { From/ } \\ \text { To } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (\mathbf{k m}) \end{array}$ | AADT | $\begin{gathered} \text { Geo- } \\ \text { metrics } \end{gathered}$ | Surf. Type | $\begin{gathered} \text { Surf. } \\ \text { Width } \end{gathered}$ | Capacity | Struct. Adeq. | Drainage |  |  |
| 01370 | KENTS MILL ROAD | PEVENSEY ROAD <br> JOLY BOUNDARY | 0.84 | 50 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | 6-10 | REC | 184,720 |
| 01380 | SCHMIDTS ROAD | KENTS MILL ROAD | 0.73 | 20 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 6-10 | None | 0 |
| 01390 | PEVENSEY ROAD | FARM VIEW ROAD <br> 1KM NORTH OF PROUDFOOT ROAD | 1.00 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | ADEQ | None | 0 |
| 01400 | PEVENSEY ROAD | 700M NORTH OF PROUDFOOT ROAD PROUDFOOT ROAD | 1.00 | 300 | ADEQ | ADEQ | ADEQ | ADEQ | NOW | 1-5 | REC | 277,569 |



## Municipal Road Appraisal - Sheet 2

|  | MAX. POINTS |  |  |  |
| :--- | :--- | :--- | :--- | ---: |
| D. POINT RATINGS | MAX. |  |  |  |
| 81. Horiz. Alignment | 10 | - | - | RATING |
| 82. Vert. Alignment | 10 | - | - | 9 |
| 83. Surface Condition | 10 | 10 | 10 | 10 |
| 84. Shoulder Width | 10 | 10 | - | 7 |
| 85. Surface Width | 15 | 15 | 25 |  |
| 86. Level of Service | 20 | 20 | 20 |  |
| 87. Str. Adequacy | 20 | 20 | 20 | 9 |
| 88. Drainage | 15 | 15 | 15 | 13 |
| 89. Maint. Demand | 10 | 10 | 10 | 5 |
| 90. Condition Rating | 100 | 100 | 100 | 53 |
| 90a. PCI Index |  |  |  | 32 |


| E. NEEDS | Existing <br> Condition | Minimum <br> Tolerable <br> Standard | Time <br> Of <br> Need |
| :--- | :---: | :--- | :--- |
| 91. Geometrics | 80 |  | 65 |
| 92. Surface Type | $\mathrm{G} / \mathrm{S}$ |  | $\mathrm{G} / \mathrm{S}$ |
| 93. Surface Width | 7.0 m |  | 5.5 |
| 94. Capacity | A | ADEQ | ADEQ |
| 95. Struct. Adequacy |  | Year | ADEQ |
| 96. Drainage |  | Year 2007 | ADEQ |
|  |  |  |  |
|  |  |  | ADEQ |
|  |  |  |  |



## G. IMPROVEMENT COST (\$ THOUSANDS)



|  |  |  |
| :--- | :---: | :---: |
| I. PCI History |  | J. Remarks |
|  | Year |  |
| 161. | PCI History |  |
| 162. | 0 |  |
| 163. | 0 |  |
| 164. | 0 |  |
| 165. | 0 |  |
| 166. | 0 |  |
| 167. | 0 |  |
| 168. | 0 |  |
| 169. | 0 |  |
| 170. |  |  |

## Appendix D Sample Structure Appraisal Form

## Municipal Bridge Appraisal - Sheet 1



## Municipal Bridge Appraisal - Sheet 2




| L. HISTORY |  |  |
| :---: | :---: | :---: |
| ENGINEERING INVESTIGATIONS | CONSTRUCTION IMPROVEMENTS |  |
| Type Year | Type | Year |
| 171. | 181. |  |
| 172. | 182. |  |
| 173. | 183. |  |
| 174. | 184. |  |
| 175 | 185. |  |
| 176. | 186. |  |
| 177. | 187. |  |
| 178. | 188. |  |
| 179. | 189. |  |
| 180. | 190. |  |

## M. Remarks

Bridge No. 2, Thomson Line, Lot 14, Conc XIII/XIV, 2.0 km West of 74 - Belmont Road, Municipality of Central Elgin:

Item 146 - Install guiderail on approaches, repair deck soffit and overlay bridge deck.

All calculations for costing, program sizing etc. are based upon the following parameters:
Table 1 Unit Costs

| Item | Unit | Cost (\$) |
| :--- | :--- | ---: |
| Excavation | $\mathrm{m}^{3}$ | 10 |
| Hot Mix Asphalt | t | 100 |
| Single Surface Treatment | $\mathrm{m}^{2}$ | 2.5 |
| Granular A | t | 13 |
| Granular B | t | 10 |
| Conc Base | $\mathrm{m}^{3}$ | 1000 |
| Conc- Curb and Gutter-place | linear m | 60 |
| Conc- Curb and Gutter-removal | linear m | 10 |
| Subdrains | linear m | 25 |
| Storm Sewer-525mm | linear m | 225 |
| Manholes | ea | 3500 |
| manhole removed | ea | 1000 |
| manholes-Adjust | ea | 1000 |
| Catch Basins | ea | 2500 |
| Catch-Basins- removed | ea | 1000 |
| Catch Basin Leads | Linear m | 150 |
| Catchbasins - adjust | ea | 1000 |
| Asphalt Planing | $\mathrm{m}^{2}$ | 2 |
| Asphalt Pulverizing | $\mathrm{m}^{2}$ | 1.5 |

All Calculations are based upon volumes, area or lengths and converted to other units as required based upon the following specific gravities derived from unit costs and weighted average widths of surfaces and platforms. Excavation calculations are based on the design road structure and existing weighted average platform and surface widths.

- specific gravity of 2.4 for Granular A
- Specific gravity of 2.1 for Granular 'B’
- specific gravity of 2.45 for HMA
- specific gravity of 2.6 for concrete

All calculations also include adjustment factors for general construction, engineering, terrain and contingency.

## Road Cross-Section Assumptions

All rural sections Assumed 500 mm ditch depth which equals $.55 \mathrm{~m} 3 / \mathrm{m}$ road length/side with a $2: 1$ side slope
$\qquad$

## Earth Roads

300 mm depth of excavation to remove unsuitable materials

## Gravel Roads

300mm depth of Granular A

## Rural LCB

150mm depth of Granular A
300mm depth of Granular B
Assumed a triple surface treatment was in place (double in year of construction, single year after)

## Rural HCB

150 mm depth of Granular A,
350mm Granular
100 mm of HMA in place

## Rural Conc

150 mm depth of Granular A;
150 mm of Granular B
150mm Conc

## SU LCB

150 mm depth of Granular A
300mm depth of Granular B
Assumed a triple surface treatment was in place (double in year of construction, single year after)

## SU-HCB

150mm depth of Granular A
350mm Granular B
100 mm of HMA in place

All urban cross-sections assume curb on both sides, sub-drain on both sides and 525 mm pipe through 60\% of the length; catchbasins and manholes every 90m

## UR LCB

150 mm depth of Granular A,
350mm Granular
3 courses of SST in place (double in year of construction, single year after)

## UR HCB

150 mm depth of Granular A, 350mm Granular B

100mm of HMA in place
UR CONC
150 mm depth of Granular A;
150mm of Granular B
150 mm of Concrete in place
$\qquad$
Township Strong
Data Last Refreshed February 18, 2009


| Section |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| No. | Road |
| Name |  |


| Section <br> No. | Road Name | $\begin{gathered} \text { From/ } / \\ \text { To } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (\mathrm{km}) \\ \hline \end{array}$ | AADT | Roadside | Speed <br> Limit | Avg. Operating Speed | Number of Deficiencies on Section |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | $\begin{gathered} \hline \text { Horz. } \\ \text { Curves } \end{gathered}$ | Horz. Stop Sight Dist | $\begin{gathered} \text { Vert. } \\ \text { Curves } \end{gathered}$ | $\begin{aligned} & \hline \text { Vert. Stop } \\ & \text { Sight Dist. } \end{aligned}$ |
| 00330 | BLACK CREEK ROAD | WEST END <br> RODEO ROAD | 0.50 | 25 | R | 80 | 80 | 0 | 0 | 0 | 3 |
| 00340 | BLACK CREEK ROAD | RODEO ROAD HIGHWAY 11 | 1.24 | 50 | R | 80 | 80 | 0 | 0 | 0 | 0 |
| 00350 | RODEO ROAD | BLACK CREEK ROAD BROOKSIDE ROAD | 2.37 | 150 | R | 80 | 75 | 2 | 0 | 0 | 1 |
| 00360 | BROOKSIDE ROAD | CHAPMAN STRONG ROAD RODEO ROAD | 2.03 | 150 | R | 80 | 80 | 0 | 0 | 0 | 4 |
| 00370 | CHAPMAN STRONG ROAD | SOUTH END BROOKSIDE ROAD | 1.06 | 120 | R | 80 | 70 | 0 | 0 | 0 | 4 |
| 00380 | CHAPMAN STRONG ROAD | BROOKSIDE ROAD <br> BLOOMFIELD ROAD | 2.01 | 100 | R | 80 | 70 | 1 | 0 | 0 | 6 |
| 00390 | BLOOMFIELD ROAD | CHAPMAN STRONG ROAD HIGHWAY 124 | 1.73 | 100 | R | 80 | 75 | 4 | 0 | 0 | 4 |
| 00400 | RODEO ROAD | BROOKSIDE ROAD HIGHWAY 124 | 2.05 | 150 | R | 60 | 60 | 0 | 0 | 0 | 7 |
| 00410 | BROOKSIDE ROAD | RODEO ROAD HIGHWAY 11 | 2.25 | 190 | R | 80 | 80 | 0 | 0 | 0 | 3 |
| 00420 | O'BRIEN ROAD | BROOKSIDE ROAD HIGHWAY 124 | 2.05 | 150 | R | 80 | 80 | 3 | 0 | 0 | 0 |
| 00430 | COTTRALL ROAD | HIGHWAY 124 ADAMS ROAD | 1.55 | 75 | R | 80 | 80 | 1 | 0 | 0 | 1 |
| 00440 | ADAMS ROAD | COTTRELLS ROAD HORNIBROOK ROAD | 0.81 | 125 | R | 80 | 80 | 0 | 0 | 0 | 0 |
| 00450 | ADAMS ROAD | HORNIBROOK ROAD <br> 700M EAST OF WEST END | 1.20 | 25 | R | 80 | 80 | 0 | 0 | 0 | 1 |
| 00460 | ADAMS ROAD | 700M EAST OF WEST END <br> WEST END | 0.70 | 5 | R | 80 | 80 | 0 | 0 | 0 | 3 |
| 00470 | HORNIBROOK ROAD | ADAMS ROAD PARKES LANE | 2.03 | 100 | R | 80 | 80 | 0 | 0 | 0 | 3 |
| 00480 | PARKES LANE | WEST END <br> HORNIBROOK ROAD | 3.05 | 60 | R | 80 | 60 | 9 | 0 | 0 | 6 |


| Section No. | Road <br> Name | From/ To | Length (km) | AADT | Roadside Env. | Speed <br> Limit | Avg. Operating Speed | Horz. Curves | Horz. Stop Sight Dist | Vert. Curves | Vert. Stop Sight Dist. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00490 | HORNIBROOK ROAD | PARKES LANE <br> MACHAR STRONG BOUNDARY ROAD WEST | $\mathrm{T}^{2.05}$ | 200 | R | 80 | 80 | 2 | 0 | 0 | 0 |
| 00500 | CHEER LAKE ROAD | MACHAR STRONG BOUNDARY ROAD WEST SOUTH END | 2.25 | 100 | R | 80 | 65 | 5 | 0 | 0 | 5 |
| 00510 | UPLANDS ROAD | MACHAR STRONG BOUNDARY ROAD WEST PARKES LANE | 2.29 | 5 | R | 80 | 30 | 9 | 0 | 0 | 9 |
| 00520 | HILL VALLEY ROAD | HORNIBROOK ROAD UPLANDS ROAD | 1.65 | 100 | R | 60 | 60 | 0 | 0 | 0 | 3 |
| 00530 | HILL VALLEY ROAD | UPLANDS ROAD ALBERT STREET | 2.42 | 100 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 00540 | PINE LANE | HILL VALLEY ROAD SOUTH END | 0.69 | 10 | R | 80 | 70 | 0 | 0 | 0 | 2 |
| 00550 | ALBERT STREET | HILL VALLEY ROAD <br> MACHAR STRONG BOUNDARY ROAD WEST | $\mathrm{T}^{2.04}$ | 100 | R | 80 | 70 | 5 | 0 | 0 | 4 |
| 00560 | TOWER ROAD | MACHAR STRONG BOUNDARY ROAD WEST HILL VALLEY ROAD | 1.99 | 200 | R | 80 | 80 | 0 | 0 | 0 | 3 |
| 00570 | TOWER ROAD | HILL VALLEY ROAD HIGH STREET | 2.08 | 200 | R | 60 | 60 | 0 | 2 | 0 | 3 |
| 00600 | BASSO ROAD | EAST SIDE OF RAILWAY TRACKS EAST END | 1.26 | 100 | R | 80 | 70 | 0 | 0 | 0 | 5 |
| 00610 | RONALD STREET | CHERYL CRESCENT BASSO ROAD | 0.54 | 50 | R | 80 | 80 | 0 | 0 | 0 | 0 |
| 00710 | LAKESHORE DRIVE | PEVENSEY ROAD <br> 170M WEST OF PEVENESY ROAD | 0.17 | 225 | R | 80 | 80 | 0 | 0 | 0 | 0 |
| 00720 | SUNNY BEACH ROAD | $\underset{\text { LAKE }}{\text { LAKESHORE DRIVE }}$ | 0.15 | 50 | R | 80 | 80 | 0 | 0 | 0 | 0 |
| 00730 | LAKESHORE DRIVE | 170M WEST OF PEVENESY ROAD SOUTH LAKE BERNARD ROAD | 0.62 | 225 | R | 80 | 80 | 1 | 0 | 0 | 0 |
| 00740 | ELIZABETH STREET | LAKESHORE DRIVE WEST END | 0.40 | 170 | R | 50 | 40 | 0 | 0 | 0 | 0 |
| 00750 | SOUTH LAKE BERNARD ROAD | LAKESHORE DRIVE GILCHRIST TRAIL | 2.93 | 50 | R | 40 | 40 | 0 | 0 | 0 | 0 |


| Section <br> No. | Road Name | $\begin{gathered} \text { From/ } / \\ \text { To } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (\mathrm{km}) \\ \hline \end{array}$ | AADT | Roadside | Speed <br> Limit | Avg. Operating Speed | Number of Deficiencies on Section |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | $\begin{gathered} \hline \text { Horz } \\ \text { Curves } \end{gathered}$ | Horz. Stop Sight Dist | $\begin{aligned} & \text { Vert. } \\ & \text { Curves } \end{aligned}$ | Vert. Stop Sight Dist. |
| 00760 | GILCHRIST TRAIL | SOUTH LAKE BERNARD ROAD TEE INTERSECTION | 0.74 | 40 | R | 80 | 60 | 2 | 0 | 0 | 2 |
| 00770 | GILCHRIST TRAIL | $\begin{aligned} & \text { SOUTH END } \\ & \text { NORTH END } \end{aligned}$ | 0.54 | 40 | R | 80 | 60 | 2 | 0 | 0 | 1 |
| 00780 | SOUTH LAKE BERNARD ROAD | GILCHRIST TRAIL FLANIGAN TRAIL | 1.83 | 50 | R | 80 | 65 | 3 | 0 | 0 | 3 |
| 00790 | SOUTH LAKE BERNARD ROAD | FLANIGAN TRAIL PIPES O PAN LANE | 3.21 | 75 | R | 80 | 60 | 8 | 1 | 0 | 6 |
| 00810 | SOUTH LAKE BERNARD ROAD | 600M WEST OF PIPES O PAN LANE CRESCENT ROAD | 0.49 | 200 | R | 40 | 40 | 0 | 0 | 0 | 0 |
| 00820 | SOUTH LAKE BERNARD ROAD | CRESCENT ROAD <br> HIGH ROCK DRIVE | 0.46 | 200 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 00830 | SOUTH LAKE BERNARD ROAD | HIGH ROCK DRIVE MUSKOKA ROAD | 1.53 | 390 | R | 60 | 60 | 0 | 0 | 0 | 3 |
| 00850 | HIGH ROCK DRIVE | SOUTH LAKE BERNARD ROAD CRESCENT ROAD | 0.42 | 390 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 00860 | HIGH ROCK DRIVE | CRESCENT ROAD BIRCH LANE | 1.27 | 390 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 00880 | BIRCH LANE | HIGH ROCK DRIVE THE POINT TRAIL | 0.33 | 120 | R | 80 | 80 | 0 | 0 | 0 | 1 |
| 00890 | ASPEN LANE | BIRCH LANE SOUTH END | 0.21 | 40 | R | 80 | 80 | 1 | 0 | 0 | 0 |
| 00900 | BIRCH LANE | ASPEN LANE <br> WEST END | 0.72 | 100 | R | 80 | 50 | 3 | 0 | 0 | 1 |
| 00910 | THE POINT TRAIL | BIRCH LANE WEST END | 0.12 | 35 | R | 80 | 50 | 2 | 0 | 0 | 1 |
| 00920 | HIGH ROCK DRIVE | BIRCH LANE 120M SOUTH OF BRIDGE | 0.78 | 390 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 00930 | HIGH ROCK DRIVE | 120M SOUTH OF BRIDGE 120M NORTH OF BRIDGE | 0.20 | 390 | R | 50 | 50 | 0 | 0 | 0 | 0 |
| 00940 | HIGH ROCK DRIVE | 120M NORTH OF BRIDGE TURTLE ROCK LANE | 2.22 | 390 | R | 60 | 50 | 4 | 0 | 0 | 3 |


| $\begin{aligned} & \text { Section } \\ & \text { No. } \\ & \hline \end{aligned}$ | Road Name | $\begin{gathered} \text { From/ } / \\ \text { To } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (\mathrm{km}) \\ \hline \end{array}$ | AADT | Roadside | Speed <br> Limit | Avg. Operating Speed | Number of Deficiencies on Section |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | $\begin{aligned} & \text { Horz } \\ & \text { Curves } \end{aligned}$ | $\begin{aligned} & \hline \text { Horz. Stop } \\ & \text { Sight Dist } \end{aligned}$ | $\begin{aligned} & \text { Vert. } \\ & \text { Curves } \end{aligned}$ | $\begin{aligned} & \hline \text { Vert. Stop } \\ & \text { Sight Dist. } \end{aligned}$ |
| 00950 | TURTLE ROCK LANE | HIGH ROCK DRIVE EAST END | 0.32 | 25 | R | 80 | 40 | 1 | 0 | 0 | 0 |
| 00960 | HIGH ROCK DRIVE | TURTLE ROCK LANE LAYOLOMI DRIVE | 0.89 | 390 | R | 60 | 55 | 1 | 0 | 0 | 0 |
| 00980 | HIGH ROCK DRIVE | LAYOLOMI DRIVE ALBERT STREET | 1.96 | 390 | R | 60 | 60 | 1 | 0 | 0 | 0 |
| 00990 | ALBERT STREET | HIGH ROCK DRIVE <br> 200M SOUTH OF SUNSET DRIVE | 0.49 | 390 | R | 50 | 50 | 0 | 0 | 0 | 0 |
| 01040 | MUSKOKA ROAD | SOUTH LAKE BERNARD ROAD SOUTH LAKE BERNARD ROAD | 0.91 | 540 | R | 60 | 60 | 1 | 0 | 0 | 1 |
| 01050 | SCHOOL HOUSE LANE | MUSKOKA ROAD WEST END | 0.19 | 30 | R | 80 | 80 | 0 | 0 | 0 | 1 |
| 01060 | MUSKOKA ROAD | SOUTH LAKE BERNARD ROAD ROBINS ROAD | 1.77 | 150 | R | 60 | 60 | 0 | 0 | 0 | 2 |
| 01070 | ROBINS ROAD | HIGHWAY 11 <br> MUSKOKA ROAD | 1.42 | 30 | R | 60 | 60 | 3 | 0 | 0 | 2 |
| 01080 | MUSKOKA ROAD | ROBINS ROAD <br> VALLEY VIEW ROAD | 2.58 | 150 | R | 60 | 60 | 0 | 1 | 0 | 1 |
| 01090 | VALLEY VIEW ROAD | HIGHWAY 11 <br> MUSKOKA ROAD | 0.72 | 50 | R | 80 | 65 | 0 | 0 | 1 | 2 |
| 01100 | MCLARENS LANE | MUSKOKA ROAD EAST END | 0.25 | 15 | R | 80 | 80 | 0 | 0 | 0 | 1 |
| 01110 | MUSKOKA ROAD | VALLEY VIEW ROAD HIGHWAY 11 | 1.14 | 150 | R | 60 | 60 | 1 | 0 | 0 | 0 |
| 01120 | MAPLE SUGAR LANE | HIGHWAY 11 NORTH END | 0.12 | 150 | R | 80 | 80 | 0 | 0 | 0 | 0 |
| 01130 | SUNNY RIDGE ROAD | HIGHWAY 11 HIGH ROCK DRIVE | 1.50 | 400 | R | 80 | 80 | 2 | 0 | 0 | 3 |
| 01150 | BUCK HAVEN ROAD | SUNNY RIDGE ROAD HIGHWAY 11 | 0.41 | 200 | R | 80 | 80 | 1 | 0 | 0 | 0 |
| 01190 | ADAMS ROAD | COTTRELLS ROAD 900M WEST OF ALBERT STREET | 2.51 | 200 | R | 80 | 60 | 5 | 0 | 0 | 9 |


| Section <br> No. | Road <br> Name | $\begin{gathered} \text { From/ } \\ \text { To } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Length } \\ (\mathrm{km}) \end{array}$ | AADT | Roadside | Speed <br> Limit | Avg. Operating Speed | Number of Deficiencies on Section |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | Horz. Curves | Horz. Stop Sight Dist | Vert. Curves | Vert. Stop Sight Dist. |
| 01200 | ADAMS ROAD | 900M WEST OF ALBERT STREET ALBERT STREET | 0.90 | 200 | R | 80 | 80 | 0 | 0 | 0 | 1 |
| 01250 | ALBERT STREET | 500M NORTH OF ADAMS ROAD <br> 700M SOUTH OF HILL VALLEY ROAD | 0.85 | 200 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 01255 | ALBERT STREET | 700M SOUTH OF HILL VALLEY ROAD HILL VALLEY ROAD | 0.70 | 200 | R | 60 | 60 | 1 | 0 | 0 | 0 |
| 01280 | HILL VALLEY ROAD | ALBERT STREET <br> TOWER ROAD | 2.04 | 100 | R | 80 | 75 | 0 | 1 | 0 | 5 |
| 01290 | HILL VALLEY ROAD | TOWER ROAD EAST END | 1.43 | 30 | R | 80 | 60 | 3 | 1 | 0 | 4 |
| 01300 | PEVENSEY ROAD | 40M WEST OF INLET CREEK ROAD 620M SOUTH OF INLET CREEK ROAD | 0.69 | 300 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 01310 | PEVENSEY ROAD | 620M SOUTH OF INLET CREEK ROAD TRUDGEONS ROAD | 1.40 | 300 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 01315 | TRUDGEONS ROAD | PEVENSEY ROAD EAST END | 1.30 | 5 | R | 80 | 20 | 4 | 0 | 0 | 6 |
| 01320 | PEVENSEY ROAD | TRUDGEONS ROAD <br> MAPLE VALLEY ROAD | 1.98 | 300 | R | 60 | 60 | 3 | 0 | 0 | 3 |
| 01330 | MAPLE VALLEY ROAD | PEVENSEY ROAD <br> WEST END | 0.90 | 10 | R | 60 | 40 | 2 | 0 | 0 | 4 |
| 01340 | PEVENSEY ROAD | MAPLE VALLEY ROAD KENTS MILL ROAD | 2.17 | 300 | R | 60 | 60 | 0 | 0 | 0 | 4 |
| 01350 | GIBBONS ROAD | PEVENSEY ROAD JOLY BOUNDARY | 0.80 | 50 | R | 80 | 60 | 2 | 0 | 0 | 3 |
| 01360 | FARM VIEW ROAD | PEVENSEY ROAD <br> WEST END | 2.45 | 50 | R | 80 | 80 | 0 | 0 | 0 | 8 |
| 01370 | KENTS MILL ROAD | PEVENSEY ROAD JOLY BOUNDARY | 0.84 | 50 | R | 60 | 60 | 2 | 0 | 0 | 0 |
| 01380 | SCHMIDTS ROAD | KENTS MILL ROAD | 0.73 | 20 | R | 60 | 60 | 0 | 0 | 0 | 0 |
| 01390 | PEVENSEY ROAD | FARM VIEW ROAD <br> 1KM NORTH OF PROUDFOOT ROAD | 1.00 | 300 | R | 60 | 60 | 0 | 0 | 0 | 0 |


$\qquad$

Appendix G PSAB 3150 Summaries by Surface Type
TOWNSHIP OF STRONG
PSAB 3150 Road Valuation






| Aggregate Valuation Summary |  |  |  |
| :---: | :---: | :---: | :---: |
| Total Current Replace Cost | $\begin{gathered} \text { Total } \\ \text { Historical Cost } \end{gathered}$ | $\begin{array}{r} \text { Accrued } \\ \text { Depreciation } \end{array}$ | $\begin{gathered} \text { Net Book } \\ \text { Value } \end{gathered}$ |
| \＄218，373 | \＄127，595 | \＄82，027 | \＄45，568 |
| \＄179，557 | \＄109，027 | \＄48，633 | \＄60，394 |
| \＄87，106 | \＄55，024 | \＄21，090 | \＄33，934 |
| \＄30，752 | \＄21，830 | \＄5，931 | \＄15，899 |
| \＄43，158 | \＄27，767 | \＄9，306 | \＄18，461 |
| \＄144，820 | \＄91，481 | \＄34，657 | \＄56，824 |
| Aggregate Valuation Summary |  |  |  |
| Total Current Replace Cost | $\begin{gathered} \text { Total } \\ \hline \text { Historical Cost } \end{gathered}$ | $\begin{array}{r} \text { Accrued } \\ \text { Depreciation } \end{array}$ | Net Book Value |
| \＄425，133 | \＄23，160 | \＄206，701 | \＄26，459 |
| Aggregate Valuation Summary |  |  |  |
| Total Current Replace Cost | $\begin{gathered} \text { Tistal } \\ \text { Historical Cost } \end{gathered}$ | $\begin{array}{r} \text { Accrued } \\ \text { Depreciation } \end{array}$ | Net Book Value |
| \＄37，346 | \＄21，284 | \＄8，514 | \＄12，770 |
| \＄13，406 | \＄7，640 | \＄3，056 | \＄4，584 |
| \＄64，349 | \＄40，649 | \＄11，178 | \＄29，470 |
| \＄70，124 | \＄39，965 | \＄15，986 | \＄23，979 |
| \＄9，944 | \＄5，667 | \＄2，267 | \＄3，400 |
| \＄5，966 | \＄3，418 | \＄1，453 | \＄1，966 |
| \＄28，727 | \＄16，459 | \＄6，995 | \＄9，464 |
| \＄1，768 | \＄1，013 | \＄430 | \＄582 |
| \＄53，035 | \＄30，385 | \＄12，914 | \＄17，471 |
| \＄36，241 | \＄16，596 | \＄9，128 | \＄7，468 |
| \＄15，911 | \＄9，296 | \＄4，416 | \＄4，881 |
| \＄15，321 | \＄6，112 | \＄3，820 | \＄2，292 |
| \＄11，344 | \＄5，195 | \＄2．857 | \＄2，338 |



|  | － | ล̀ | $\stackrel{\text { }}{ }$ | ल | － | $\stackrel{\text { ® }}{ }$ |  |  |  |  |  | む | N | จ | ～ | ＊ | ๙ | ๙ | ๙ | ® | $\stackrel{\infty}{\infty}$ | － | ำ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \％\％ | q | q | q | q | q | q |  | －\％\％ |  |  | \％\％\％ | q | \％ | \＆ | q | q | q | q | q | q | q | q | q | q |
|  |  |  | $\begin{aligned} & \text { 侖 } \\ & \text { 荡 } \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\partial} \\ & \dot{G} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{0} \\ & \stackrel{\omega}{\infty} \\ & \dot{\phi} \end{aligned}$ |  |  |  |  |  |  | $\begin{aligned} & \stackrel{\rightharpoonup}{\stackrel{\rightharpoonup}{9}} \\ & \stackrel{y}{\oplus} \end{aligned}$ | $\begin{aligned} & \text { 영 } \\ & \text {. } \end{aligned}$ | $\begin{aligned} & \text { 呂 } \\ & \stackrel{0}{6} \end{aligned}$ |  | $\begin{aligned} & \hat{e} \\ & \stackrel{0}{6} \end{aligned}$ | $\begin{gathered} \infty \\ \underset{W}{\mp} \\ \underset{W}{7} \end{gathered}$ | $\begin{aligned} & \frac{8}{8} \\ & \stackrel{8}{\bullet} \\ & \stackrel{\rightharpoonup}{i \theta} \end{aligned}$ | $\stackrel{0}{\stackrel{0}{i}}$ | $\begin{aligned} & \mathscr{M} \\ & \text { M } \\ & \text { on } \end{aligned}$ | $\begin{aligned} & \text { \& } \\ & \stackrel{8}{8} \\ & \stackrel{6}{i n} \end{aligned}$ |  | $\cong$ | － |
|  |  | 啻 | \％ | 앙 | 悶 | \％ |  |  |  |  |  | 房 | \％ | \％ | \％ | \％\％ | \％ | \％ | \％ | \％ | 品 | 呂 | \％ | $\stackrel{\infty}{8}$ |
|  |  | 兼 | 僉 | \％ | 罝 | 畣 |  | － |  |  |  | \％ | \％ | 動 | 哭 | \％ | 产 | 产 | 吾 | 㐫 | ® | $\stackrel{\text { ® }}{\text { ¢ }}$ | ® | ® |




|  |  |  |  |  | $\begin{aligned} & \tilde{0} \\ & \stackrel{\omega}{\infty} \\ & \stackrel{N}{\infty} \end{aligned}$ | $\begin{aligned} & \text { H } \\ & \stackrel{\rightharpoonup}{0} \\ & \text { on } \end{aligned}$ |  | $\begin{aligned} & \tilde{(\tilde{0}} \\ & \dot{\omega} \\ & \text { in } \end{aligned}$ | $\begin{aligned} & \text { F } \\ & \stackrel{F}{6} \\ & \stackrel{0}{2} \end{aligned}$ |  |  |  | $\begin{aligned} & \stackrel{\circ}{0} \\ & \stackrel{y}{\circ} \\ & \hline \end{aligned}$ | ¢ |  |  | $\begin{aligned} & \text { ⿹\zh26灬 } \\ & \text { 呺 } \end{aligned}$ |  |  | $\begin{aligned} & \infty \\ & \stackrel{\infty}{\infty} \\ & \stackrel{\infty}{\infty} \end{aligned}$ | $\begin{aligned} & \circ .8 \\ & 0 \\ & 0 \\ & \vdots \end{aligned}$ |  | $\begin{gathered} \text { N } \\ \text { in } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { O} \\ & \text { O} \\ & \text { N } \end{aligned}$ | $\stackrel{\text { \％}}{\substack{\text { ® }}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | － |  |  | $\begin{aligned} & \text { 尔 } \\ & \stackrel{y y y y}{*} \end{aligned}$ |  |  | $\begin{aligned} & \text { 啇 } \\ & \text {. } \end{aligned}$ | $\begin{aligned} & \text { N } \\ & \stackrel{y}{*} \\ & \stackrel{\sim}{心} \end{aligned}$ | $\begin{aligned} & \text { © } \\ & \stackrel{⿷ 匚 ⿱ 艹 ⿸ ⿻ 一 丿 口 ⿴ 囗 十 ~}{\prime} \end{aligned}$ |  |  | $\stackrel{\cong}{N}$ | $\begin{aligned} & \stackrel{\circ}{i} \\ & \stackrel{\rightharpoonup}{0} \\ & \stackrel{\rightharpoonup}{i} \end{aligned}$ |  | $\begin{aligned} & \text { 槀 } \\ & \stackrel{y}{=} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{8} \\ & \text { © } \\ & \text { on } \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{0} \\ & \text { on } \\ & \text { in } \end{aligned}$ | $\begin{aligned} & \text { 等 } \\ & \text { 岕 } \end{aligned}$ |  |  | $\begin{aligned} & \text { 毋. } \\ & \underset{y y y}{*} \end{aligned}$ |  |  | － |
|  |  | $\begin{aligned} & \text { O} \\ & \text { en } \\ & \text { Non } \end{aligned}$ |  | － | $\begin{aligned} & \hat{\circ} \\ & \stackrel{e}{i n} \\ & \stackrel{e}{6} \end{aligned}$ |  |  |  | $\begin{aligned} & \stackrel{\circ}{\dot{\circ}} \\ & \stackrel{\rightharpoonup}{\dot{\theta}} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\stackrel{0}{6}} \\ & \stackrel{\oplus}{\dot{\theta}} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{8} \\ & \stackrel{8}{8} \\ & \stackrel{y}{8} \end{aligned}$ |  |  | $\begin{aligned} & \stackrel{\circ}{0} \\ & \stackrel{\circ}{\circ} \\ & \dot{\circ} \end{aligned}$ |  | $\begin{gathered} \text { N. } \\ \stackrel{\leftrightarrow}{6} \\ \hline \end{gathered}$ | $\begin{aligned} & \text { 俞 } \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ |  |  |  |  |  | $\begin{aligned} & \stackrel{\circ}{n} \\ & \stackrel{N}{i} \end{aligned}$ |  | 哭 |
|  |  |  |  |  | $\begin{aligned} & \text { 䋗 } \end{aligned}$ |  | $\begin{aligned} & \stackrel{e}{\circ} \\ & \stackrel{\rightharpoonup}{0} \\ & \stackrel{y}{0} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\stackrel{\rightharpoonup}{0}} \\ & \stackrel{\rightharpoonup}{\dot{\theta}} \end{aligned}$ |  |  | $\begin{aligned} & \infty \\ & \stackrel{0}{0} \\ & \underset{\sim}{\infty} \end{aligned}$ |  | 兑 |  |  | $\begin{aligned} & \text { ※口 } \\ & \text { N } \\ & \text { N } \end{aligned}$ |  |  |  |  |  |  | $\begin{aligned} & \text { o. } \\ & \text { ob } \\ & \text { on } \end{aligned}$ | $\stackrel{\otimes}{\text { ® }}$ | 鹤 |




 Gravel \＆Stone：Semi－urban

00260 NORTH HORN LAKE ROAD 00270 RODEO ROAD
 00290 SOUTH HORN LAKE ROAD 00300 RODEO ROAD

00310 NORTH HORN LAKE ROAD

|  |  |  |  |  |  | $\begin{aligned} & \stackrel{\rightharpoonup}{\bar{o}} \\ & \stackrel{y}{i} \end{aligned}$ |  | $\begin{gathered} \circ \\ \stackrel{\circ}{W} \\ \stackrel{\rightharpoonup}{3} \\ \hline \end{gathered}$ | $\begin{aligned} & \text { 略 } \end{aligned}$ |  | $\frac{0}{2}$ | $\begin{aligned} & \text { 等 } \\ & \text { 符 } \end{aligned}$ | $\begin{aligned} & \text { 䝹 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \text { 产 } \\ & \text { 咅 } \end{aligned}$ | $\begin{aligned} & \overline{\overleftarrow{y y}} \\ & \stackrel{\tilde{M}}{\dot{W}} \end{aligned}$ |  |  |  | $\begin{aligned} & \stackrel{\rightharpoonup}{N_{\tilde{N}}} \\ & \substack{0} \end{aligned}$ | $\begin{aligned} & \text { 瀹 } \\ & \hline \end{aligned}$ |  | \％ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\stackrel{\text { 离 }}{\text { 雨 }}$ |  | $\frac{\bar{E}}{\dot{\omega}}$ |  | 㕏 |  |  |  |  | $\begin{aligned} & \text { 喜 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 㡈 } \end{aligned}$ | $\begin{aligned} & \text { \& } \\ & \text { 世ín } \end{aligned}$ | $\begin{aligned} & \text { 膏 } \\ & \stackrel{\rightharpoonup}{6} \end{aligned}$ |  | $\begin{aligned} & \stackrel{e}{6} \\ & \stackrel{y}{6} \end{aligned}$ | $\stackrel{\circ}{\circ}$ | $\begin{aligned} & 8 \\ & \stackrel{8}{8} \\ & 0 \end{aligned}$ | $\begin{aligned} & \text { 䧺 } \\ & \stackrel{\rightharpoonup}{\bar{j}} \end{aligned}$ |  |  | $\begin{aligned} & \text { 滷 } \end{aligned}$ | $\begin{aligned} & \text { 瞢 } \\ & \hline \end{aligned}$ | $\stackrel{\text { N }}{\stackrel{\rightharpoonup}{\omega}} \underset{\bar{\omega}}{5}$ | $\frac{\stackrel{0}{5}}{\frac{1}{i n}}$ | $\stackrel{\text { 萼 }}{\text { ¢ }}$ |
|  |  | $\begin{aligned} & \text { 瞉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \stackrel{\Sigma}{\tilde{j}} \end{aligned}$ |  | 啇 |  |  | $\begin{aligned} & \text { 蠋 } \end{aligned}$ |  |  |  | $\begin{aligned} & \stackrel{\circ}{6} \\ & \stackrel{\rightharpoonup}{6} \\ & \hline \dot{\omega} \end{aligned}$ | $\stackrel{o}{F_{i n}^{2}}$ | $\begin{aligned} & \text { 曾 } \\ & \frac{\stackrel{\rightharpoonup}{\bar{j}}}{2} \end{aligned}$ | 高 | $\begin{aligned} & \text { N } \\ & \text { 管 } \end{aligned}$ | $\begin{gathered} \text { 咇 } \\ \text { Wig } \end{gathered}$ | $\begin{aligned} & \text { 采 } \\ & \stackrel{\rightharpoonup}{6} \end{aligned}$ | $\begin{aligned} & \text { 喜 } \\ & \text { 举 } \end{aligned}$ |  |  |  | $\begin{aligned} & \text { ig } \\ & \stackrel{\circ}{\circ} \mathrm{O} \end{aligned}$ | 罵 |
|  |  |  | $\begin{aligned} & \stackrel{\rightharpoonup}{⿳ 亠 丷 ⿵ 冂 ⿱ 丷 口 犬} \\ & \stackrel{\rightharpoonup}{6} \end{aligned}$ |  |  |  |  | $\begin{aligned} & \stackrel{\infty}{\kappa} \\ & \stackrel{\text { Non }}{\omega} \end{aligned}$ |  |  | $\begin{gathered} \text { 憲 } \\ \text { 品 } \end{gathered}$ |  | $\begin{aligned} & \stackrel{\circ}{6} \\ & \stackrel{\circ}{6} \end{aligned}$ |  | $\begin{aligned} & \text { 坒 } \\ & \text { Wi } \end{aligned}$ |  |  | $\begin{aligned} & \text { 噥 } \end{aligned}$ | $\begin{aligned} & \text { 滷 } \end{aligned}$ |  | $\begin{aligned} & \text { 眔 } \end{aligned}$ |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | $=$ | $=$ | $\stackrel{\square}{\square}$ | $\stackrel{\square}{\square}$ | $\stackrel{\square}{\square}$ | $\cdots$ | $\stackrel{\square}{\square}$ | $\stackrel{\square}{\square}$ |  | ® | $=$ | \％ | ๕ | － | $\stackrel{\square}{\square}$ | $\stackrel{\sim}{\sim}$ | － | む | $\stackrel{\rightharpoonup}{*}$ | $\stackrel{\sim}{\sim}$ | \％ | 8 | $\stackrel{ \pm}{*}$ |
|  |  | \％ | \％ | \％ | \％ | \％ | \％ | \％ | \％ | 8 | \％ | \％ | 8 | \％ | \％ | \％ | \％ | \％ | \％ | \％ | ¢ | \％ | \％ | \％ |
|  |  | $\begin{gathered} \text { 喆 } \end{gathered}$ | $\begin{aligned} & \text { 关 } \\ & \stackrel{\rightharpoonup}{5} \end{aligned}$ | $\begin{aligned} & \text { 管 } \\ & \stackrel{i}{i} \end{aligned}$ |  |  | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \text { 長 } \end{aligned}$ |  | $\begin{aligned} & \text { öixy } \\ & \stackrel{ֻ \pi}{6} \end{aligned}$ |  |  | $\begin{aligned} & \text { g. } \\ & \stackrel{8}{\dot{8}} \end{aligned}$ | $\frac{\stackrel{o}{z}}{\stackrel{\rightharpoonup}{i}}$ | $\begin{aligned} & \text { 亳 } \\ & \stackrel{\rightharpoonup}{\bar{\omega}} \end{aligned}$ | $\stackrel{\circ}{\circ}$ | $\begin{aligned} & \text { N } \\ & \text { 管 } \end{aligned}$ |  |  |  |  |  |  | $\begin{gathered} \text { 总 } \\ \stackrel{\rightharpoonup}{6} \end{gathered}$ | 嘉 |
|  |  | 产 | 产 | 咢 | 总 | \％ | 気 | 器 | 管 | 器 | 器 | 要 |  | 䉞 | $\stackrel{\square}{\square}$ | 哭 | 皆 | 膏 | \％ | \％ | 管 | 熨 |  | 莒 |
|  |  | 黑 | 畐 | \％ | \％ | 咢 | 動 | 梁 | \％ | 動 | 重 | 总 |  | \＄ | \％ | 畐 | 馬 | \％ | \％ | 先 | 总 | ¢ | 置 | \％ |
| 皆 | ㅍ | 跉 | 枵 | \％ | \％ | $\stackrel{\circ}{\text { \％}}$ | \％ig | － | \％ | \％ | \％ | 骪 | ¢ \％ | 율 | ¢ | \％ | $\stackrel{\square}{8}$ | 吕 | $\stackrel{\infty}{\sim}$ | $\stackrel{\infty}{\sim}$ | \％ | $\stackrel{\circ}{\bar{\circ}}$ | ¢ู | $\stackrel{\infty}{\infty}$ |
|  |  | ® | $\stackrel{\circ}{\circ}$ | \％ | $\stackrel{\circ}{\circ}$ | F | － | － | $\because$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | 8 | － | $\stackrel{\circ}{\circ}$ | 品 | $\therefore$ | 品 | $\stackrel{\circ}{\circ}$ | ¿ | 古 |
|  |  | $\stackrel{\sim}{\circ}$ | $\bigcirc$ | \％ | ¢ | ni | $\stackrel{\circ}{\circ}$ | $\stackrel{\sim}{\circ}$ | $\bigcirc$ | $\stackrel{\circ}{\infty}$ | \％ | ¢ | 8 | ¢ | \％ | $\stackrel{\infty}{\sim}$ | $\stackrel{\circ}{\infty}$ | $\bigcirc$ | \＆ | $\stackrel{\square}{0}$ | ： | $\stackrel{\circ}{\circ}$ | ¢ | ＊ |
|  |  | \％ | $\underset{\sim}{ \pm}$ | ¢ | $\stackrel{\square}{\text { ® }}$ | $\stackrel{\square}{\square}$ | － | $\stackrel{1}{2}$ | $\stackrel{\square}{\text { a }}$ | $\stackrel{\sim}{\sim}$ | 号 | 吕 | 戸 | $\stackrel{\square}{9}$ | $\%$ | $\stackrel{\square}{\text { \％}}$ | $\stackrel{\square}{\circ}$ | ～ั | \％ | 南 | ® | 喜 | \％ | \％ |


| Gravel \＆Stone：Rural |  |
| :---: | :---: |
| Section | Road Name |
| 00320 | Rodeo road |
| 00330 | BLACK CREEK ROAD |
| 00340 | BLACK CREEK ROAD |
| 00350 | RODEO ROAD |
| 00360 | BROOKSIDE ROAD |
| 00370 | CHAPMAN STRONG ROAD |
| 00380 | CHAPMAN STRONG ROAD |
| 00390 | BLOOMFIELD ROAD |
| 00400 | RODEO ROAD |
| 00410 | BROOKSIDE ROAD |
| 00420 | O＇brien road |
| 00430 | COTTRALL ROAD |
| 00440 | ADAMS ROAD |
| 00450 | ADAMS ROAD |
| 00460 | ADAMS ROAD |
| 00470 | HORNIBROOK ROAD |
| 00480 | parkes lane |
| 00500 | CHEER LAKE Road |
| 00540 | PINE LANE |
| 00550 | Albert street |
| 00600 | basso road |
| 00610 | RONALD STREET |
| 00730 | LAKESHORE DRIVE |
| 00740 | ELIZABETH STREET |

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|  | $\stackrel{\text { ̇ }}{\sim}$ | ～ | $\stackrel{\sim}{\sim}$ | \％ | ¢ | N | $\bar{\sim}$ | ำ | ำ | － | $\cdots$ | $\cong$ | － | $\stackrel{\infty}{\sim}$ | ๙ | $\pm$ | む | ～ | $\stackrel{ }{\sim}$ | $\stackrel{\text { ® }}{ }$ | ～ | $\stackrel{ }{ }$ | $\stackrel{ }{ }$ | $\stackrel{\circ}{\circ}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ¢ ¢ \％ |  | q | \％ | q | q | q | q | q | q | q | q | q | q | q | q | q | q | q | \％ | q | q | q | q | q |
|  |  | \％ | 㖪 |  |  |  |  | $\begin{aligned} & \text { 誌 } \\ & \text { Nemon } \end{aligned}$ |  |  |  | $\begin{aligned} & \text { ్లo } \\ & \tilde{\sim} \\ & \hline \end{aligned}$ | $\begin{aligned} & \approx \\ & \stackrel{\circ}{\tilde{N}} \\ & \stackrel{y}{\aleph} \end{aligned}$ | $\begin{aligned} & \frac{J}{N} \\ & \stackrel{N}{\infty} \\ & \stackrel{y}{\infty} \end{aligned}$ |  | $\begin{aligned} & \frac{\pi}{N} \\ & \stackrel{N}{i} \end{aligned}$ | $\begin{aligned} & \text { \% } \\ & \stackrel{0}{0} \\ & \stackrel{\circ}{6} \end{aligned}$ | $\begin{aligned} & \ddot{\circ} \\ & \stackrel{y}{\infty} \\ & \hline \end{aligned}$ | $\begin{aligned} & \mathscr{g} \\ & \stackrel{y}{6} \\ & \frac{0}{6} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\rightharpoonup}{\dot{a}} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathbf{\omega}} \\ & \stackrel{\rightharpoonup}{\Phi} \\ & \hline \end{aligned}$ | $\begin{aligned} & \tilde{\sim} \\ & \stackrel{\sim}{0} \\ & \stackrel{\sim}{i} \end{aligned}$ | $\begin{aligned} & \text { 若 } \\ & \stackrel{8}{8} \end{aligned}$ | \％ |
|  | \％ | \％ | 器 | ¢ | 㐌 | 薄 | 褭 | \％．\％ | $\begin{aligned} & \text { g. } \\ & \hline 0.0 \end{aligned}$ | \％ |  | 骨 | $\stackrel{\circ}{\circ}$ | $\stackrel{\otimes}{\substack{8 \\ \hline}}$ | \％ | $\begin{aligned} & \text { ®. } \\ & \text { on } \end{aligned}$ | $\stackrel{\circ}{8}$ | 鬲 | 原 | $\stackrel{\tilde{\circ}}{\circ}$ | $\stackrel{\circ}{!}$ | 举 | 兴 | \％ |
| － |  |  | ® | $\stackrel{\bar{\sim}}{\sim}$ | ¢ | \％ | ® | $\stackrel{\text { ® }}{\sim}$ | 睘 | $\stackrel{\text { ® }}{\sim}$ | 合 | ® | $\stackrel{\text { ® }}{\text { ® }}$ | \％ | ¢ | ※ | 蜕 | 梁 | 茴 | 墨 | 涫 | 茴 | 逼 | ® |
| ¿ | $\stackrel{\infty}{\sim}$ | $\stackrel{\infty}{\sim}$ | ¢ | $\stackrel{\square}{\square}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | $\stackrel{\infty}{\sim}$ | 呙 | $\stackrel{¢}{\text { ¢ }}$ | $\stackrel{¢}{\text { ¢ }}$ | 瑟 | $\stackrel{\sim}{\circ}$ | ¢¢ | $\stackrel{\square}{\circ}$ | \％ | $\stackrel{\otimes}{\stackrel{\circ}{i}}$ | $\stackrel{\sim}{\sim}$ | $\stackrel{\infty}{\sim}$ | $\stackrel{\infty}{\sim}$ | ¢ | ＋${ }_{\text {¢ }}^{\text {＋}}$ | $\stackrel{\text { ¢ }}{\substack{\text { c }}}$ | 尔 | 尔 | ัู |
|  | $\stackrel{\circ}{\circ}$ | $\bigcirc$ | $\bigcirc$ | $\stackrel{\circ}{\circ}$ | $\bigcirc$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\square}{+}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{0}{0}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\circ}{\circ}$ | $\infty$ | $\because$ |
|  | $\stackrel{\sim}{\sim}$ | $\infty$ | $\infty$ | $\hat{N}$ | $\stackrel{\circ}{\infty}$ | $\stackrel{\circ}{\infty}$ | $\bigcirc$ | $\stackrel{\circ}{\circ}$ | $\bigcirc$ | $\stackrel{\circ}{\circ}$ | 号 | $\bigcirc$ | $\stackrel{\circ}{i}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\sim}{\sim}$ | $\stackrel{\square}{6}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{セ}{\sim}$ | $\stackrel{9}{\sim}$ | $\underbrace{\infty}_{\infty}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\sim}{\sim}$ | ¢ | $\stackrel{\circ}{\circ}$ |
|  | $\stackrel{\text { ® }}{\sim}$ | $\stackrel{\cong}{\underline{¢}}$ | $\stackrel{\text { ¢ }}{ }$ | ？ | ！ | \％ | ㄷ．． | No | $\stackrel{\sim}{\circ}$ | \％ | $\stackrel{\square}{\circ}$ | 찯 | ฐ | $\stackrel{\otimes}{\sim}$ | N | ®ٌ000 | $\stackrel{ \pm}{+}$ | $\stackrel{\square}{\circ}$ | $\stackrel{\square}{\sim}$ | $\stackrel{\circ}{\circ}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | $\stackrel{\text { a }}{\text { N }}$ | \％ | $\stackrel{\square}{4}$ |


| Gravel \＆Stone：Rural |  |
| :--- | :--- |
| Section | Road Name |
| 00750 | sOUTH LAKE BERNARD ROAD |
| 00780 | sOUTH LAKE BERNARD ROAD |
| 00790 | sOUTH LAKE BERNARD ROAD |
| 000810 | SOUTH LAKE BERNARD ROAD |
| 00820 | SOUTH LAKE BERNARD ROAD |
| 00880 | BIRCH LANE |
| 00890 | ASPEN LANE |
| 00900 | BIRCH LANE |
| 00910 | THE POINT TRALL |
| 00950 | TURTLE ROCK LANE |
| 01050 | SCHOOL HOUSE LANE |
| 01060 | MUSKOKA ROAD |
| 01070 | ROBINS ROAD |
| 01080 | MUSKOKA ROAD |
| 01090 | VALLEY VIEW ROAD |
| 01100 | MCLARENS LANE |
| 01110 | MUSKOKA ROAD |
| 01120 | MAPLE SUGAR LANE |
| 01190 | ADAMS ROAD |
| 01200 | ADAMS ROAD |
| 01250 | ALBERT STREET |
| 01280 | HILL VALLEY ROAD |
| 01290 | HILL VALLEY ROAD |
| 01310 | PEVENSEY ROAD |


| Dimensions |  |  | ${ }^{\text {Index }}$ | Road Base |  |  |  |  | Road Surtace |  |  |  | Aggregate Valuation Summary |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\underset{\substack{\text { Length } \\(\mathrm{km})}}{\text { chem }}$ | Platfrom Width (m) | Surface Width (m) | PCI | $\begin{gathered} \text { Year } \\ \text { Const. } \end{gathered}$ | $\begin{aligned} & \text { Deflation } \\ & \text { Index } \end{aligned}$ | $\begin{array}{r} \text { Historic } \\ \text { Const. Cost } \end{array}$ | Stire | $\begin{aligned} & \text { Useful Life } \\ & \text { Remaining } \end{aligned}$ | $\underset{\substack{\text { Year } \\ \text { Const. } \\ \hline}}{ }$ | $\begin{aligned} & \text { Deflation } \\ & \text { Index } \end{aligned}$ | $\begin{array}{rc}\text { Historic } & \text { Life } \\ \text { Const. Cost } & \text { Span }\end{array}$ | Useful Life Remaining | Total Current Replace Cos | $\begin{gathered} \text { Total } \\ \text { Historical Cost } \end{gathered}$ | $\begin{array}{r} \text { Accrued } \\ \text { Depreciation } \end{array}$ | Net Book value |
| 1.98 | 9.0 | ${ }_{6} .0$ | 87.2 | 1998 | 643 | \$214,799 | 40 | 30 |  |  |  |  | ¢33,863 | S214,799 | s53,70 | \$161,099 |
| 0.90 | 6.5 | 5.5 | 49.6 | 1983 | 0.399 | ${ }_{545,238}$ | 40 | 15 |  |  |  |  | \$113,395 | \$45,238 | \$28,274 | \$16,964 |
| 2.17 | 6.0 | 5.0 | 44.6 | 1981 | 0.353 | s90,073 | 40 | 13 |  |  |  |  | \$254,909 | 990,073 | \$60,799 | \$29,274 |
| 0.80 | 6.0 | 5.0 | 59.0 | 1986 | 0.458 | 543,34 | 40 | 18 |  |  |  |  | 993,976 | \$43,034 | \$2,669 | \$19,365 |
| 2.45 | 7.0 | 6.0 | 59.0 | 1986 | 0.458 | \$150,920 | 40 | 18 |  |  |  |  | \$329,572 | \$150,920 | s83,06 | 867, |
| 0.84 | 8.0 | 6.0 | 73.8 | 1992 | 0.570 | 572.561 | 40 | ${ }^{24}$ |  |  |  |  | \$127,318 | \$72,561 | \$29,024 | \$4,3,57 |
| 0.73 | 8.0 | 6.0 | 54.5 | 1985 | 0.431 | \$47,968 | 40 | 17 |  |  |  |  | \$110,645 | \$47,966 | \$27,425 | \$20,271 |
| 1.00 | 9.0 | 6.0 | 872 | 1998 | 0.643 | \$108,484 | 40 | 30 |  |  |  |  | \$188,618 | \$108,884 | \$27,121 | \$81,363 |
| Dimensions |  |  | Index | Road Base |  |  |  |  | Road Surface |  |  |  | Aggregat Valuation Summary |  |  |  |
| $\underset{\substack{\text { Lengin } \\(\mathrm{mm})}}{ }$ | Platfrom Width (m) | Surface Width (m) | PCI | $\begin{gathered} \text { Yeart } \\ \text { const. } \end{gathered}$ | $\begin{aligned} & \text { Deflation } \\ & \text { Index } \end{aligned}$ | $\begin{array}{r} \text { Historic } \\ \text { Const. Cost } \end{array}$ | Lile <br> Span | Useful Life Remaining | $\begin{aligned} & \text { Year } \\ & \text { Const. } \end{aligned}$ | $\begin{aligned} & \text { Deflation } \\ & \text { Index } \end{aligned}$ | $\begin{array}{rc}\text { Historic } & \text { Life } \\ \text { Const. Cost } & \text { Span }\end{array}$ | Useful Life Remaining | Total Current Replace Cost | $\begin{gathered} \text { Total } \\ \text { Historical Cost } \end{gathered}$ | $\begin{array}{r} \text { Accrued } \\ \text { Depreciation } \end{array}$ |  |
| 229 | 3.5 | 2.5 | -21.5 | 1968 | 0.129 | \$4,502 | 40 | 0 |  |  |  |  | \$39,780 | \$4,502 | \$4,502 | so |
| 1.30 | 4.0 | ${ }^{3} 0$ | -21.5 | 1988 | 0.129 | \$2,865 | 40 | 0 |  |  |  |  | ${ }_{\text {s22,133 }}$ | ${ }_{52} 2865$ | ${ }^{2} 2865$ | so |


| Gravel \& Stone: Rural |  |
| :--- | :--- |
| Section | Road Name |
| 01320 | PEVENSEY ROAD |
| 01330 | MAPLE VALLEY ROAD |
| 01340 | PEVENSEY ROAD |
| 01350 | GIBBONS ROAD |
| 01360 | FARM VIEW ROAD |
| 01370 | KENTS MILL ROAD |
| 01380 | SCHMIDTS ROAD |
| 01390 | PEVENSEY ROAD |
| Earth: | Rural |
| Section | Road Name |
| 00510 | UPLANDS ROAD |
| 01315 | TRUDGEONS ROAD |

Strong Township<br>Facility Reviews

TSH 36-19679

September 8, 2008

Location: Sundridge Works Garage


Original Construction: 1980 (estimated)

Facility Attributes: Facility is a combination of combustible and non-combustible construction in fair condition.
Summary Observations: the structure is within 25 years of replacement. The framed spaces in the north bay include staff areas below and an office on the mezzanine. Reviews of the structural frame are recommended every three years. Site area is available for expansion.

Replacement Cost: \$430,000
*includes design, engineering and construction; excludes equipment, furnishings, land, approvals and GST. All amounts are 2008 dollars.

Budget for Repairs / Replacements for next 20 years: $\$ 172,500$

Following are comments on major facility systems based on review of the site August 22, 2008.

## Site

The facility addresses one street. Land is available for potential expansion.

## Structure

The pre-engineered steel frame was generally found to be in fair condition. The wood frame component including the mezzanine is also in fair condition. In general it is recommended that the structural steel frame be reviewed by a structural engineer every three years. The floor slab is generally in fair to good condition. The replacement horizon for the above-grade steel structure, the concrete foundations and floor slab is projected at 25 years.

## Envelope

Metal roofing is in good condition and within 20 years of needing replacement. Windows and doors are in fair condition within 10 years of replacement. Replacement of operating and weather sealing components for the
overhead doors is recommended within 5 years. Replacement of the section overhead doors is recommended in 10 years.

## Interior

Finishes are minimal for the interior. The wood frame mezzanine in the north bay is painted wall board and is recommended to be repainted every 7 years.

The staff spaces and office are recommended to have doors and windows upgraded to 45 minute rated closures.

The floor slab is recommended to be resealed within 5 years. Ceiling tiles are recommended to be replaced when repainting is done. Washroom accessories are due for replacement within 5 years.

## Mechanical

Heating includes electric baseboard units in the staff areas. Natural gas fired suspended radiant heaters provide heat for the garage bays and are reported to be functioning and approximately 15 years old. Natural gas fired hot water heaters are provided at to north and west ends of the garage bays. The exterior portion of the flue vent for the south heater is turned downward and should be repositioned to an upright position. A Carbon monoxide detection system is recommended to be installed for the garage in general. Exhaust fans in the garage are functional and are recommended to be replaced in the next 5 years.

Potable water is supplied from a well adjacent to the garage. The washroom facilities drain to an on site septic system.

## Electrical

Main switch and distribution components are estimated at 25 years old and due for replacement within 10 years.

## Summary of Priorities and Costs Projected for the Next 20 years

Items are categorized according to priority:

```
Essential - Life Safety
Required - maintaining facility / operations
Desired - upgrade facility / operations
```

Mechanical - reposition water heater flue vent to upright position $\underline{500}$

Sub-total - Essential Items 2,500

Required

| Structural - detailed review every 3 years (total amount over 20 years) | $\$ 14,000$ |
| :--- | ---: |
| Envelope - replace operating components and weather seals for overhead doors | 3,000 |
| Interior - reseal concrete floors | 2,000 |
| Interior - replace doors, windows and frames between staff areas and garage |  |
| with fire rated closures. Include separation of stair from garage area with direct | 28,000 |
| access to the exterior. | 40,000 |
| Mechanical - radiant heating system repairs / replacements | 12,000 |
| Mechanical - replace exhaust fans | 20,000 |
| Electrical - service replacement | $\$ 119,000$ |
| Sub-total - Required Items | 50,000 |
| Desired | 50,500 |

## End

Strong Township
Facility Reviews

TSH 36-19679

September 8, 2008

## Location: Sand Storage Dome



## Original Construction: 1993

The one storey storage dome is 15 years old.

Facility Attributes: The structure is of combustible and non-combustible construction and in fair condition. The envelope is wood sheathing and asphalt shingle. The facility includes at-grade access for vehicles and is not designed for barrier-free access.

Summary Observations: the shed is within 25 years of replacement.

Replacement Cost: $\$ 330,000$
*includes design, engineering and construction; excludes equipment, furnishings, land, approvals and GST. All amounts are 2008 dollars.

## Budget for Repairs / Replacements for next 20 years: \$150,000

Following are comments on major facility systems based on review of the site August 22, 2008.

## Site

The facility addresses one street. The facility is not designed for expansion.

## Structure

Timber framing, concrete support walls and pilasters are in fair to good condition. Vertical hairline cracks were observed in the west half of the foundation wall. Structural inspection is recommended for every three years. The replacement horizon is projected at 25 years.

## Envelope

Roofing is asphalt shingle and also serves generally as the exterior cladding. The roofing will be due for replacement within 10 years. The entry framing and roofing show greater wear and will be due for repair and roofing replacement within 5 years.

## Interior

The interior is exposed framing, sheathing and concrete in fair to good condition.

## Mechanical

No mechanical services.

## Electrical

A sub-panel fed from the Works Garage provide power for lighting in the Dome. The panel and lighting fixtures appear to be in fair condition.

## Summary of Priorities and Costs Projected for the Next 20 years

Items are categorized according to priority:

Essential - Life Saftey
Required - maintaining facility / operations
Desired - upgrade facility / operations

## Essential

No items

## Required

Structural - detailed review every 3 years (total amount over 20 years)
\$12,000

Exterior - repairs to entry framing and replacement of roofing for the entry only 8,000

Exterior - asphalt shingle and roofing paper replacement. Includes an allowance for replacement of $10 \%$ of the roof sheathing.

130,000

Sub-total - Required Items \$150,000

## Desired

No items
End

Strong Township<br>Facility Reviews<br>TSH 36-19679

September 8, 2008

Location: Storage Shed


Original Construction: 2008

The one storey storage shed was constructed this year.

Facility Attributes: The shed structure includes two compartments on different levels and is of combustible and non-combustible construction and in good condition. The envelope is prefinished metal. The facility includes atgrade access for vehicles and is not designed for barrier-free access.

Summary Observations: the shed is within 35 years of replacement.

Replacement Cost: $\$ 80,000$
*includes design, engineering and construction; excludes equipment, furnishings, land, approvals and GST. All amounts are 2008 dollars.

## Budget for Repairs / Replacements for next 20 years: $\$ 14,000$

Following are comments on major facility systems based on review of the site August 22, 2008.

## Site

The facility addresses two streets. The facility includes area adjacent to allow for expansion.

## Structure

Timber framing and concrete support walls are in good condition. Structural inspection is recommended for every three years. The replacement horizon is projected at 35 years.

## Envelope

Roofing and siding are pre-finished metal in good condition. Overhead doors are in good condition.

## Interior

The interior is exposed framing, siding and concrete in good condition.

## Mechanical

No mechanical services.

## Electrical

A sub-panel fed from the Works Garage provides power for lighting in the Shed. The panel and lighting fixtures appear to be in good condition.

## Summary of Priorities and Costs Projected for the Next 20 years

Items are categorized according to priority:

Essential - Life Saftey
Required - maintaining facility / operations
Desired - upgrade facility / operations

## Essential

No items

## Required

Structural - detailed review every 3 years (total amount over 20 years)

Exterior - replacement of door operating components

Sub-total - Required Items
\$14,000

## Desired

No items

## End

Map 1 Roads by Surface Type







[^0]:    Friday，February 20， 2009

